

Planning Proposal to amend the Orange Environmental Plan 2011

Leeds Parade, Clergate

Traffic Impact Assessment

September 2021

Reference: 213 rep 210903 final

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Contact

Website: www.amberorg.com.au



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1. Introduction

Amber Organisation has been engaged by Premise Australia Pty Ltd to provide a preliminary traffic impact assessment associated with a proposal to amend the Orange Local Environmental Plan 2011 with respect to land formerly occupied by the Orange Abattoir at the northern end of Leeds Parade, Clergate.

A Planning Proposal has previously been submitted for the site which resulted in an amendment to the Orange Local Environmental Plan 2011 (LEP) rezoning the subject land from RU1 – Primary Production and IN1 – General Industrial to R5 – Large Lot Residential and E4 – Environmental Living. A master plan for the site was developed with a lot yield of approximately 450 lots and a minimum lot size of 4,000 square metres. The master plan also provided a draft internal road layout with access to the site proposed from Leeds Parade. It is understood the master plan was a conceptual model for delivery of the overarching development of the land and was not intended to represent the final development arrangement.

This Planning Proposal involves further amendment to the LEP which would reduce the minimum lot size and increase the yield of the land to approximately 700 lots. In addition, the proposal introduces an upgrade of a private level crossing to a public level crossing to provide access via Clergate Road in the west and introduces a third access via Pearce Lane in the north.

This report has been prepared to provide a preliminary traffic impact assessment of the proposal sufficient to support the planning proposal and provide Council, regulatory agencies and the Department of Planning, Industry and Environment the confidence that the proposal can be delivered with acceptable impacts to the local traffic environment.

2. Transport Environment

2.1 Site Location

The site is located on land formerly occupied by the Orange Abattoir at the northern end of Leeds Parade, Clergate. The subject land consists of the following five lots and has an area of approximately 290 hectares:

- Lot 15 DP6694, 390 Clergate Road, Orange
- Lot 3 DP255983, 440 Clergate Road, Orange
- Lot 2 DP255983, 440 Clergate Road, Orange
- Lot 14 DP6694, 440 Clergate Road, Orange
- Lot 25 DP6694, 440 Clergate Road, Orange

Figure 1 shows the location of the site in relation to the surrounding transport network.

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Figure 1: Site Location

Source: OpenStreetMap



The site is currently zoned R5 – Large Lot Residential and E4 – Environmental Living following the adoption of the amendment to the LEP as part of the previous Planning Proposal. The land use zoning for the site and the surrounding area is illustrated within Figure 2.



Figure 2: Land Use Zoning

Source: NSW Government ePlanning Spatial Viewer

The surrounding land use is primarily agricultural with the following key activities provided within the surrounding area:

- The site is located approximately 5 kilometres north of the Orange CBD and 1.8 kilometres from the North Orange Shopping Centre;
- The Charles Sturt University, Orange Campus, is located approximately 1.1 kilometres southeast of the site; and
- Clergate Public School is located 2.9 kilometres north of the site.

The site is bounded to the north by Pearce Lane, to the west by the Main Western Railway Line and to the south and east by private land.

Figure 3 shows an aerial photograph view of the site and the surrounding area. The figure shows that the site is currently occupied by agricultural land and includes the Orange Abattoir.

Figure 3: Aerial Photograph



Source: SixMaps

Primary access to the site is from Leeds Parade which connects with Northern Distributor Road to the south. Leeds Parade transitions into a sealed driveway at the site which formerly provided heavy vehicle access to the abattoir buildings.

Historically abattoir staff are understood to have accessed the abattoir via a single lane bridge over the Main Western Railway Line, which staff used to walk over after parking on land on the western side of the railway line. The bridge is accessed from private land owned by the Applicant.

The site also currently benefits from a single lane railway crossing from Clergate Road and a number of gateway accesses to Pearce Lane in the north.

Figure 4 shows the south-western corner of the site and the three current accesses into the property in this area.



Figure 4: Existing Site Accesses in the Southern Portion of the Site

Source: Premise

2.2 Road Network

The primary access to the site is proposed via Leeds Parade which connects with Northern Distributor Road allowing excellent links to the Orange CBD via Leeds Parade. Northern Distributor Road also provides access to the North Orange Shopping Centre, and links with the State road network to provide access to nearby towns. Additional connections to the road network are also proposed via Clergate Road and Pearce Lane.

Leeds Parade is classified as a municipal collector road under the care and management of Orange City Council. It runs in a north-south alignment between the site boundary and Dalton Street. Between Northern Distributor Road and the Orange University access it has a carriageway width of approximately 7.0 metres which accommodates one lane of traffic in each direction. North of the university access it has a reduced carriageway width of approximately 6.5 metres with no central linemarking. South of Northern Distributor Road it has carriageway width of approximately 10 metres accommodating one lane of traffic in each direction and sealed shoulders. It has a speed limit of 80km/hr which is reduced to 50km/hr within the vicinity of the university and the built up residential and industrial areas within Orange.

Northern Distributor Road is a municipal arterial road under the care and management of Orange City Council. It provides a loop around the northern side of Orange extending between Mitchell Highway and Forbes Road. It acts as a key link between the North Orange Shopping Centre and the eastern and western extents of Orange. Within the vicinity of Leeds Parade it has a carriageway width of approximately 12 metres which accommodates one lane of traffic in each direction. It has a speed limit of 80km/hr east of Leeds Parade and a speed limit of 70km/hr west of Leeds Parade. The intersection of Leeds Parade and Northern Distributor Road is controlled by a roundabout. Hanrahan Place provides a fifth leg to the intersection to the northeast which has a length of approximately 180 metres and provides access to several commercial/industrial properties.

Clergate Road is a municipal local road that extends in a north-south alignment between Burrendong Way and Northern Distributor Road. It runs to the west of the Main Western Railway Line before continuing as Pearce Lane for a short period near the north-western corner of the site, then continues north on the eastern side of the railway line. It has a typical carriageway width of approximately 7 metres and accommodates two-way vehicle movement. It has a speed limit of 50km/hr between Northern Distributor Road and Quartz Street which increases to 80km/hr at Industry Drive and 100km/hr further north.

The intersection of Clergate Road and Northern Distributor Road is priority controlled with vehicles exiting Clergate Road provided with Give Way signage and associate line marking.

Pearce Lane is a local road which runs in an east-west alignment between Ophir Road and Clergate Road. It has a typical sealed carriageway width of approximately 6.5 metres between its two connections with Clergate Road, and an unsealed carriageway width of approximately 6 metres east of Clergate Road.

2.3 Traffic Volumes

Due to the restrictions imposed by the NSW Government in response to the COVID-19 pandemic at the time of preparing this assessment, traffic surveys have been unable to be undertaken as they would not be reflective of typical operating conditions. In order to determine the existing traffic volumes on the road network data has been extracted from the Orange Strategic Transport Model. The model provides traffic volumes for 2018 based on collected survey data and estimates the traffic volumes in 2028. The model is described within the Orange Strategic Transport Model Update Report prepared by Stantec, dated September 2018.

Data from the model has been extracted for the surrounding intersections for the year 2018 and 2028 and is provided within Appendix A. The data has also been provided visually within Figure 5.

The data suggests that Northern Distributor Road accommodates relatively high traffic volumes, with Clergate Road and Telopea Way also recording relatively high traffic volumes. These volumes reflect the road classifications with Northern Distributor Road acting as an arterial road and Clergate Road and Telopea Way acting as collector roads that provide access to the North Orange Shopping Centre and adjacent residential and industrial uses.

Traffic volumes on Pearce Lane and Dawson Gates Road are low, with moderate traffic volumes recorded on Burrendong Way.

The traffic model volumes suggest that all roads within the vicinity of the site are expected to experience modest traffic growth between 2018 and 2028.

Rosedale Gardens Traffic Calculations

Leeds Parade, Orange



2.4 Public Transport

The public transport services within the vicinity of the site are shown within Figure 6. The map indicates the only existing bus service provided within the vicinity of the site is Route 67 which travels between the Orange University Campus along Leeds Parade to the Orange CBD.





Source: Orange Buslines

2.5 Pedestrians and Cyclists

A shared path is provided along the majority of the western side of Leeds Parade between Dalton Street and the university, and a shared path is provided along the southern side of Northern Distributor Road west of Leeds Parade that links with the wider pedestrian / bicycle pathway network to the west.

2.6 Road Safety

Amber has conducted a review of the TfNSW Centre for Road Safety Crash and Casualty Statistics database for all injury crashes within the following search area:

- Northern Distributor Road between Leeds Parade and Clergate Road;
- Leeds Parade between Northern Distributor Road and the site;
- Clergate Road between Northern Distributor Road and Pearce Lane;
- The full length of Pearce Lane; and
- The relevant intersections.

The crash database provides the location and severity of all injury and fatal crashes for the fiveyear period from 2015 to 2019. The crash search revealed one moderate injury crash midblock on Clergate Road near Industry Drive when a vehicle lost control within the road carriageway.

The crash search indicates that there are no discernible crash trends within the surrounding road network. As such, it is concluded that the road network is currently operating in a relatively safe manner.



3. Development Proposal

3.1 Existing Site Approval

A Planning Proposal has previously been prepared for the site which resulted in an amendment to the LEP resulting in the rezoning of the subject land from RU1 – Primary Production and IN1 – General Industrial to R5 – Large Lot Residential, E4 – Environmental Living, RE1 – Public Recreation and SP2 - Infrastructure. A master plan for the site was developed which showed approximately 450 lots with a minimum lot size of 4,000 square metres. The master plan provided a draft internal road layout with access to the site proposed from Leeds Parade.

3.2 The Proposal

This Planning Proposal involves an amendment to the LEP which would reduce the minimum lot size and increase the yield of the land to approximately 700 lots. Exceedance of this limit would not be possible without further amendments to the LEP. Any further LEP amendment would require consultation with regulatory agencies including TfNSW and their Rail Infrastructure Manager.

The site currently has one legal road access, being south via Leeds Parade. It is expected this would be the primary point of access for vehicles travelling to and from the Orange CBD and industrial areas and for vehicles travelling east and west from Orange via the Northern Distributor Road.

In addition, the proposal introduces an upgrade of a private level crossing to a public level crossing to provide access via Clergate Road and introduces a third access via Pearce Lane which are described below:

- A new western access is proposed across the Main Western Railway line via an existing private level crossing to be upgraded to an active level crossing. The access is expected to be utilised by residents making local trips to the North Orange Shopping Centre, via Farrell Road, or travelling to the North Orange residential/industrial areas;
- A new access is proposed in the north-western corner of the site via Pearce Lane. Vehicles travelling in this direction would be expected to predominantly be travelling north (such as to Clergate School), or to Burrendong Way to travel north towards Mullion Creek, Lake Burrendong or Wellington. A small proportion of these vehicles located in the very northern extent of the subdivision may also travel via Clergate Road to the North Orange Shopping Centre, via Farrell Road, or travelling to the North Orange residential/industrial areas.

Given the connectivity afforded by the Northern Distributor Road and Leeds Parade, it is expected that the majority of vehicles would utilise the Leeds Parade access.

Figure 7 shows the locations of the proposed accesses in relation to the surrounding road network and North Orange Shopping Centre, with a masterplan for the site provided within Appendix B.





Figure 7: Site Master Plan

Source: Premise



4. Traffic Assessment

4.1 Traffic Generation

The Roads & Maritime Services Technical Direction 04a: Guide to Traffic Engineering Developments -Updated Traffic Surveys, dated August 2013, has been used to estimate the traffic generation of the site. The traffic generation rates for low density residential land use within regional areas are as follows:

- Daily vehicle trips: 7.4 movements per dwelling;
- Weekday average morning peak hour vehicle trips: 0.78 movements per dwelling; and
- Weekday average evening peak hour vehicle trips: 0.71 movements per dwelling.

Application of the above rates to the 700 residential lots results in a future traffic generation of 5,180 vehicle movements per day, and 546 and 497 vehicle movements (two-way total) in the morning and evening peak hours respectively.

4.2 Trip Distribution

It is typical for residential activities to yield a trip distribution involving about 80% of traffic in the morning peak hour being departing trips, and 20% arriving trips. Similarly, it is typical that 30% of trips will be departing and 70% will be arriving trips in the evening peak hour. As such, the site is expected to generate the following traffic volumes during the morning and afternoon peak periods.

	AM Peak (vph)	PM Peak (vph)
Arriving Trips	109	348
Departing Trips	437	149
Total	546	497

Table 1: Site Peak Hour Traffic Generation

The site is provided with three connections to the road network which includes Leeds Parade, Clergate Road and Pearce Lane. The following provides a breakdown of the distribution of site traffic at the accesses:

- It has been assumed that 60% of traffic will utilise Leeds Parade which provides good access to the Orange CBD and Northern Distributor Road for the majority of the site;
- It has been assumed that 30% of traffic will utilise the Clergate Road access which will primarily be used by residents making local trips to the North Orange Shopping Centre via Farrell Road; and
- It has been assumed that 10% of traffic will utilise the northern access to Pearce Lane which will be utilised by some residents travelling north such as to Clergate School or to Burrendong Way. A small proportion of residents located in the very northern extent of the subdivision may also utilise the access to travel south via Clergate Road.

The distribution of site traffic on the road network is shown within Figure 8.

Rosedale Gardens Traffic Calculations

Leeds Parade, Orange Site Traffic



A

Clergate Road

8

4.3 Traffic Assessment

In order to determine the traffic impact generated by the subdivision an analysis of the operation of the surrounding intersections was carried out using the SIDRA computer modelling program. The concepts of intersection capacity and level of service, as defined in the guidelines published by the RTA (2002), are discussed in Appendix C together with criteria for their assessment. The assessment of the level of service for sign-controlled intersections is based on the average delay (seconds/vehicle) of the critical movement and the assessment for signalised intersections is the average delay over all movements at the intersection.

The intersections that have been assessed have been identified where a significant increase in traffic is expected based on the traffic distribution outlined within Figure 8. These intersections include:

- Leeds Parade / Northern Distributor Road
- Clergate Road / Northern Distributor Road
- Clergate Road / Farrell Road
- Farrell Road / Telopea Way
- Telopea Way / Northern Distributor Road
- Clergate Road / Site Access (middle access)
- Pearce Lane / Clergate Road / Site Access (northern access)

The traffic volumes used for the assessment have been based on the traffic volumes presented within Section 2.3 which are taken from the Orange Strategic Model for the year 2028. It is considered appropriate to utilise these volumes given survey data is unable to be collected at this time. Further, the model provides an estimate of the future traffic volumes on the road network which is when the site is likely to be constructed and the associated traffic volumes will be accommodated on the road network.

A future scenario has also been assessed which adds the development traffic to the strategic model 2028 traffic volumes. The future scenario traffic volumes are shown in Figure 9.

The results of the analysis are provided within Appendix D and are summarised below.



Rosedale Gardens Traffic Calculations

Leeds Parade, Orange

Orange Traffic Model



A

Clergate Road

4.3.1 Leeds Parade / Northern Distributor Road

The roundabout intersection has been modelled based on the existing lane configuration. The results of the SIDRA analysis for the morning peak hour for the intersection of Leeds Parade, Hanrahan Place and Northern Distributor Road are summarised in Table 2.

		Strategi	c Model Traf	fic 2028	Future Traffic Conditions 2028		
Movement		Average Delay (sec)	95% Queue (m)	Level of Service	Average Delay (sec)	95% Queue (m)	Level of Service
	Left	5.6		А	6.2		А
Leeds Parade	Through	4.9	0.1	А	5.2	0.6	А
South	Right	9.4	9.1	А	9.7	9.0	А
	Right-Right	11.8		В	12.1		В
	Left-Left	4.7		Α	5.3		А
Northern	Left	4.6	8.6	Α	5.7	10.8	A
Road East	Right	9.5		Α	10.4		В
	Right-Right	10.7		В	11.6		В
	Left-Left	9.4	5.1	Α	11.9		В
Hanrahan	Left	8.5		Α	11.2		В
Place	Right	12.9		В	15.3	0.3	В
	Right-Right	15.3		В	17.7		В
	Left-Left	7.5		Α	8.0		Α
Leeds Parade	Left	8.1	2.0	Α	8.8	14.0	Α
North	Through	6.9	3.8	Α	7.4	14.9	A
	Right Turn	11.9		В	12.7		В
	Left-Left	5.9		Α	6.2		A
Northern	Left	5.4	20.0	Α	5.8	20.4	Α
Road West	Right	9.9	30.9	А	10.8	39.4	В
	Right-Right	11.1		В	12.0		В

Table 2: SIDRA Analysis Results Summary – AM Peak 2028

The SIDRA analysis for the AM peak indicates the following:

- The intersection is expected to continue to operate with minimal queue lengths on all legs of the intersection. The longest queue is recorded on the western leg of Northern Distributor Road which increases from 30.9 metres to 39.4 metres;
- The overall average delay at the intersection increases by 1.2 seconds which represents a minimal change. All legs of the intersection experience a minor delay that is well within the acceptable operating conditions; and
- The intersection is expected to continue to operate with a good level of service.

The results of the SIDRA analysis for the evening peak hour for the intersection of Leeds Parade, Hanrahan Place and Northern Distributor Road are summarised in Table 3.

	Movement		c Model Traf	fic 2028	Future Traffic Conditions 2028		
Mover			95% Queue (m)	Level of Service	Average Delay (sec)	95% Queue (m)	Level of Service
	Left	6.3		А	6.7		А
Leeds Parade	Through	6.1		А	6.2		А
South	Right	10.6	8.2	В	10.7	8.9	В
	Right-Right	13.0	-	В	13.0	-	В
	Left-Left	4.9		А	5.1		А
Northern	Left	5.9	13.8	А	6.5	15.9	А
Distributor Road East	Right	10.1		В	10.5		В
	Right-Right	11.3		В	11.7	-	В
	Left-Left	7.7	6.2	А	8.3		А
Hanrahan	Left	7.5		А	8.2		А
Place	Right	11.8		В	12.4	1.0	В
	Right-Right	14.2	-	В	14.8		В
	Left-Left	6.7		А	6.8		А
Leeds Parade	Left	6.7		А	6.8	10.4	А
North	Through	6.3	6.9	А	6.4	10.4	Α
	Right Turn	11.3	-	В	11.5	-	В
	Left-Left	4.5		Α	5.2		А
Northern	Left	4.0	16.0	Α	4.7	20 5	А
Road West	Right	8.6	16.9	Α	9.3	20.5	А
	Right-Right	9.8	1	А	10.5		В

Table 3: SIDRA Analysis Results Summary - PM Peak 2028

The SIDRA analysis for the PM peak indicates the following:

- The intersection is expected to continue to operate with minimal queue lengths on all legs of the intersection. The longest queue is recorded on the western leg of Northern Distributor Road which increases from 16.9 metres to 20.5 metres;
- The overall average delay at the intersection increases by 0.4 seconds which represents a minimal change. All legs of the intersection experience a minor delay that is well within the acceptable operating conditions; and
- The intersection is expected to continue to operate with a good level of service.

Overall, the intersection with the 2028 Orange Strategic Model volumes is shown to be operating with ample spare capacity to accommodate an increase in traffic resulting in the intersection operating with a good level of service. As a result, the moderate increase in traffic generated by the site traffic only results in minor increases to queue lengths and delays. The AM and PM peak

hour degree of saturation for the intersection with the development traffic is 0.615 and 0.431, respectively, which indicates there would still be ample spare capacity even with the development traffic.

4.3.2 Clergate Road / Northern Distributor Road

The intersection has been modelled based on the existing lane configuration which provides a right turn lane from Northern Distributor Road and separate right and left turn lanes from Clergate Road. Vehicles exiting Clergate Road are provided with Give Way restrictions.

The results of the SIDRA analysis for the morning peak hour for the intersection of Clergate Road and Northern Distributor Road are summarised in Table 4.

Movement		Strategi	c Model Traf	fic 2028	Future Traffic Conditions 2028		
		Average Delay (sec)	95% Queue (m)	Level of Service	Average Delay (sec)	95% Queue (m)	Level of Service
Northern	Through	0.0	13.9	А	0.1	45 5	А
East	Right Turn	10.9		В	11.6	15.5	В
Clorgate Boad	Left Turn	8.4	664.5	А	8.7	921.8	А
Clergate Road	Right Turn 710.9	710.9		F	1266.5		F
Northern	Left Turn	5.7		А	5.7	0.0	А
West	Through	0.1	0.0	А	0.1		А

 Table 4: SIDRA Analysis Results Summary - AM Peak 2028

The SIDRA analysis for the AM peak indicates the intersection generally operates with acceptable queue lengths and delays except for the right turn movement from Clergate Road which has reached capacity even without the development traffic. Once the movement reaches capacity the reported queue lengths and delays increase exponentially and are not reportedly accurately by the model.

The results of the SIDRA analysis for the evening peak hour for the intersection of Clergate Road and Northern Distributor Road are summarised in Table 5.

Movement		Strategi	c Model Traf	fic 2028	Future Traffic Conditions 2028		
		Average Delay (sec)	95% Queue (m)	Level of Service	Average Delay (sec)	95% Queue (m)	Level of Service
Northern	Through	0.1	9.6	А	0.1	13.9	А
East	Right Turn	8.2		А	9.7		А
Clargata Boad	Left Turn	7.3		А	7.9	613.4	А
	Right Turn	529.7	411.5	F	1044.7		F
Northern	Left Turn	5.6		А	5.7	0.0	А
West	Through	0.1	0.0	А	0.1		А

Table 5: SIDRA Analysis Results Summary – PM Peak 2028

The SIDRA analysis for the PM peak is similar to the AM peak and indicates the intersection generally operates with acceptable queue lengths and delays except for the right turn movement from Clergate Road which has reached capacity.

The traffic volumes are based on the Orange Strategic Traffic Model which estimates traffic volumes based on vehicles moving between an origin and a destination. The level of complexity of the model does not account for changes in behaviour generated by certain routes taking a significantly longer time due to delays at intersections such as what is being reported above for Clergate Road and Northern Distributor Road. In reality vehicles are likely to choose an alternative route in order to travel westbound on Northern Distributor Road once delays become unacceptable or an alternative route is available with a shorter travel time.

4.3.3 Clergate Road / Farrell Road

The intersection has been modelled based on the existing lane configuration which provides one lane of traffic in all directions with no dedicated turn facilities. Vehicles exiting Farrell Road are provided with Stop restrictions.

The results of the SIDRA analysis for the morning peak hour for the intersection of Clergate Road and Farrell Road are summarised in Table 6.

Movement		Strategi	c Model Traf	fic 2028	Future Traffic Conditions 2028			
		Average Delay (sec)	95% Queue (m)	Level of Service	Average Delay (sec)	95% Queue (m)	Level of Service	
Clergate Road	Left Turn	5.6	0.0	А	5.6	0.0	А	
South	Through	0.1		А	0.1		А	
Clergate Road	Through	0.2	1.0	А	1.0	10.0	А	
North	Right Turn	7.7	1.9	Α	8.1		Α	
	Left Turn	10.9	10.0	В	11.4	13.5	В	
raffell Road	Right Turn	15.9	10.8	С	19.7		С	

 Table 6: SIDRA Analysis Results Summary - AM Peak 2028

The SIDRA analysis for the AM peak indicates the following:

- The intersection is expected to continue to operate with minimal queue lengths on all legs of the intersection;
- The overall average delay at the intersection increases by 0.8 seconds; and
- The intersection is expected to continue to operate with a good level of service on the Clergate Road legs of the intersection and an acceptable level of service on the Farrell Road leg of the intersection.

The results of the SIDRA analysis for the evening peak hour for the intersection of Clergate Road and Farrell Road are summarised in Table 7.

Movement		Strategi	c Model Traf	fic 2028	Future Traffic Conditions 2028			
		Average Delay (sec)	95% Queue (m)	Level of Service	Average Delay (sec)	95% Queue (m)	Level of Service	
Clergate Road	Left Turn	5.6	0.0	А	5.6	0.0	А	
South	Through	0.1		А	0.1		А	
Clergate Road	Through	1.1	10.0	А	1.6	18.0	А	
North	Right Turn	7.8	13.0	А	8.3		А	
	Left Turn	9.9	1.0	Α	10.4	4.3	В	
	Right Turn	16.6	1.9	С	19.4		С	

Table 7: SIDRA Analysis Results Summary - PM Peak 2028

The SIDRA analysis for the PM peak indicates the following:

- The intersection is expected to continue to operate with minimal queue lengths on all legs of the intersection;
- The overall average delay at the intersection increases by 0.9 seconds; and
- The intersection is expected to continue to operate with a good level of service on the Clergate Road legs of the intersection and an acceptable level of service on the Farrell Road leg of the intersection.

Overall, the intersection is expected to operate with acceptable queue lengths and delays. The delay for vehicles turning from Farrell Road is moderate with all other legs of the intersection operating with minimal delays. The degree of saturation for the intersection with the development traffic is 0.369 and 0.396 during the morning and evening peak hour, respectively, which suggests there is capacity at the intersection to accommodate an increase in traffic.

4.3.4 Farrell Road / Telopea Way

The intersection has been modelled as a network with the intersection of Telopea Way and Northern Distributor Road given both intersections are signalised and the phasing for the intersections has been linked. The model is based on the existing lane configuration.

SCATS data has been provided by Transport for NSW for the intersection. The phasing provided has been adopted for the intersection however, the phase times have been optimised to account for the increase in traffic.

The results of the SIDRA analysis for the morning peak hour for the intersection of Farrell Road and Telopea Way are summarised in Table 8.

Movement		Strategi	c Model Traf	fic 2028	Future Traffic Conditions 2028			
		Average Delay (sec)	95% Queue (m)	Level of Service	Average Delay (sec)	95% Queue (m)	Level of Service	
Telopea Way	Through	3.6	5.8	А	3.7	8.4	А	
South	Right Turn	14.7	12.4	В	7.0	5.4	А	
	Left Turn	14.2	10.6	В	15.4	13.9	В	
Farrell Road	Right Turn	28.0	2.1	С	29.2	8.3	С	
Telopea Way	Left Turn	28.3	3.1	С	28.6	4.7	С	
North	Through	16.5	13.1	В	16.2	14.5	В	

Table 8: SIDRA Analysis Results Summary - AM Peak 2028

The SIDRA analysis for the AM peak indicates the following:

- The intersection is expected to continue to operate with moderate queue lengths on all legs of the intersection. The existing turn facilities are able to accommodate the expected queue lengths;
- The overall average delay at the intersection decreases by 0.6 seconds as a result of the traffic distribution and associated changes to the phase time. The only notable change to the delay at the intersection is the increase in delay to right turning vehicles from the southern leg of Telopea Way which increases by 7.7 seconds from 7.0 seconds to 14.7 seconds;
- The intersection is expected to continue to operate with a good or acceptable level of service on all legs of the intersection; and
- The degree of saturation at the intersection is expected to increase from 0.398 to 0.438.

The results of the SIDRA analysis for the evening peak hour for the intersection of Farrell Road and Telopea Way are summarised in Table 9.

Movement		Strategi	c Model Traf	fic 2028	Future Traffic Conditions 2028			
		Average Delay (sec)	95% Queue (m)	Level of Service	Average Delay (sec)	95% Queue (m)	Level of Service	
Telopea Way	Through	4.3	11.7	А	3.3	11.6	А	
South	Right Turn	20.9	10.2	С	27.3	13.6	С	
Forroll Bood	Left Turn	12.9	6.4	В	18.7	10.8	В	
Falleli Roau	Right Turn	22.7	3.6	С	28.9	6.8	С	
Telopea Way	Left Turn	22.8	3.8	С	29.5	10.0	С	
North	Through	15.5	20.2	В	13.5	26.1	В	

Table 9: SIDRA Analysis Results Summary - PM Peak 2028

The SIDRA analysis for the PM peak indicates the following:

- The intersection is expected to continue to operate with moderate queue lengths on all legs of the intersection. The existing turn facilities are able to accommodate the expected queue lengths;
- The overall average delay at the intersection is expected to increase by 2.1 seconds, with minor increases in delay experienced on all legs of the intersection;
- The intersection is expected to continue to operate with a good or acceptable level of service on all legs of the intersection; and
- The degree of saturation at the intersection is expected to increase from 0.592 to 0.597.

Overall, the intersection is expected to continue to operate in an acceptable manner with only minor increases in queue lengths and delays generated by site traffic.

4.3.5 Telopea Way / Northern Distributor Road

The intersection has been modelled as a network with the intersection of Telopea Way and Farrell Road given both intersections are signalised and the phasing for the intersections has been linked. The model is based on the existing lane configuration.

SCATS data has been provided by Transport for NSW for the intersection. The phasing provided has been adopted for the intersection however, the phase times have been optimised to account for the increase in traffic.

The results of the SIDRA analysis for the morning peak hour for the intersection of Telopea Way and Northern Distributor Road are summarised in Table 10.

Movement		Strategi	c Model Traf	fic 2028	Future Traffic Conditions 2028		
		Average Delay (sec)	95% Queue (m)	Level of Service	Average Delay (sec)	95% Queue (m)	Level of Service
Northern	Through	4.2	15.1	Α	4.4	18.2	А
East	Right Turn	29.4	9.2	С	31.0	16.5	С
	Left Turn	17.4	16.0	В	17.6	17.4	В
теюреа way	Right Turn	27.6	15.5	С	30.6	19.6	С
Northern	Left Turn	9.6	10.5	А	9.6	10.5	А
Distributor Road West	Through	16.3	59.1	В	17.5	63.9	В

Table 10: SIDRA Analysis Results Summary – AM Peak 2028

The SIDRA analysis for the AM peak indicates the following:

- The intersection is expected to continue to operate with moderate queue lengths on all legs of the intersection. The existing turn facilities are able to accommodate the expected queue lengths;
- The overall average delay at the intersection is expected to increase by 1.3 seconds, with minor increases in delay experienced on all legs of the intersection;
- The intersection is expected to continue to operate with a good or acceptable level of service on all legs of the intersection; and

• The degree of saturation at the intersection is expected to increase from 0.753 to 0.780.

The results of the SIDRA analysis for the evening peak hour for the intersection of Telopea Way and Northern Distributor Road are summarised in Table 11.

Movement		Strategi	c Model Traf	fic 2028	Future Traffic Conditions 2028			
		Average Delay (sec)	95% Queue (m)	Level of Service	Average Delay (sec)	95% Queue (m)	Level of Service	
Northern	Through	6.6	36.7	А	5.9	39.9	А	
Distributor Road East	Right Turn	27.0	20.3	С	29.4	26.4	С	
	Left Turn	11.4	13.4	В	12.0	17.2	В	
теюреа мау	Right Turn	19.8	13.8	В	20.0	17.0	С	
Northern	Left Turn	10.7	11.3	В	11.7	13.7	В	
Distributor Road West	Through	17.1	30.3	В	18.7	42.6	В	

Table 11: SIDRA Analysis Results Summary - PM Peak 2028

The SIDRA analysis for the PM peak indicates the following:

- The intersection is expected to continue to operate with moderate queue lengths on all legs of the intersection. The existing turn facilities are able to accommodate the expected queue lengths. It is noted that the through movement travelling eastbound on Northern Distributor Road experiences an increase in queue length of 12.3 metres which is equivalent to approximately two cars;
- The overall average delay at the intersection is expected to increase by 0.8 seconds, with minimal increases in delay experienced on all legs of the intersection;
- The intersection is expected to continue to operate with a good or acceptable level of service on all legs of the intersection; and
- The degree of saturation at the intersection is expected to decrease from 0.764 to 0.736 due to the distribution of traffic at the intersection with the site traffic and associated changes to the phase times.

Overall, the intersection is expected to continue to operate in an acceptable manner with only minor increases in queue lengths and delays generated by site traffic.

4.3.6 Clergate Road / Site Access (Middle Access)

The central access location proposed for the site is currently a private access that experiences a minimal level of traffic. As such, an assessment of the intersection has not been undertaken for the existing operation of the intersection.

The results of the SIDRA analysis for the morning and evening peak hours for the intersection of Clergate Road and the central site access are summarised in Table 12. Both roads are provided with one lane of traffic in each direction with no turn lanes provided on either road. Give Way restrictions are applied to vehicles exiting the site.

	AM Futu	ire Traffic Co	onditions	PM Future Traffic Conditions			
Movement		Average Delay (sec)	95% Queue (m)	Level of Service	Average Delay (sec)	95% Queue (m)	Level of Service
Clergate Road	Through	0.1	0.7	А	0.1	2.4	А
South	Right Turn	5.6	0.7	А	5.6	2.4	А
Site Access	Left Turn	5.7	2.7	А	5.6	0.0	А
	Right Turn	5.8	2.1	А	6.0	0.9	А
Clergate Road North	Left Turn	5.6	0.0	А	5.6	0.0	Α
	Through	0.0	0.0	А	0.0	0.0	А

The results of the SIDRA analysis indicate the following:

- The intersection is expected to operate with minimal queue lengths on all legs of the intersection;
- The average delay at the intersection is 4.5 seconds during both the AM and PM peak; and
- The intersection is expected to operate with a good level of service.

Overall, the intersection is expected to operate in a suitable manner with minimal delays and queue lengths, and has ample spare capacity to accommodate an increase in traffic.

4.3.7 Pearce Lane / Clergate Road / Site Access (Northern Access)

The intersection has been modelled as a cross-type intersection however, the ultimate intersection is likely to be established as a staggered T-intersection which provides a safer road alignment. Each leg of the intersection was modelled with one lane of traffic in each direction and no turn facilities. Give Way restrictions were applied to the eastern Pearce Lane leg of the intersection and the site access as the primary traffic movement is likely to be vehicles travelling north-south between the western leg of Pearce Lane and Clergate Road.

Based on discussions with Orange City Council no vehicles associated with the site have been assumed to be travelling along Pearce Lane to/from Ophir Road.

The results of the SIDRA analysis for the morning peak hour for the intersection of Pearce Lane, Clergate Road and the site access are summarised in Table 13.

	Strategi	c Model Traf	fic 2028	Future Traffic Conditions 2028			
Movem	Average Delay (sec)	95% Queue (m)	Level of Service	Average Delay (sec)	95% Queue (m)	Level of Service	
	Left Turn	5.6		А	5.7		А
Site Access	Through	4.3	0.1	А	4.4	0.9	А
	Right Turn	5.6		А	5.7		А
Pearce Lane East	Left Turn	5.6		А	5.6		А
	Through	4.2	0.4	А	4.2	0.4	А
	Right Turn	5.6		А	5.8		А
	Left Turn	5.6		А	5.6		А
Clergate Road	Through	0.0	0.0	А	0.0	0.0	А
	Right Turn	5.5		А	5.5		А
Pearce Lane West	Left Turn	5.6		А	5.6		А
	Through	0.0	0.3	Α	0.1	0.5	Α
	Right Turn	5.5		А	5.5		А

Table 13: SIDRA Analysis Results Summary - AM Peak 2028

The SIDRA analysis for the AM peak indicates the following:

- The intersection is expected to continue to operate with minimal queue lengths on all legs of the intersection;
- The overall average delay at the intersection increases by 0.8 seconds; and
- The intersection is expected to continue to operate with a good level of service.

The results of the SIDRA analysis for the evening peak hour for the intersection of Pearce Lane, Clergate Road and the site access are summarised in Table 14.

Table 14. SIDBA	Analysis	Doculte	Summary	/ _ DM	Dook 2028
Table 14: SIDRA	Alldiysis	Results	Summary		Fedr 2020

	Strategi	c Model Traf	fic 2028	Future Traffic Conditions 2028			
Movemo	Average Delay (sec)	95% Queue (m)	Level of Service	Average Delay (sec)	95% Queue (m)	Level of Service	
	Left Turn	5.6		А	5.6		А
Site Access	Through	4.3	0.1	А	4.4	0.3	А
	Right Turn	5.6		А	5.8		А
Pearce Lane East	Left Turn	5.6		А	5.7	0.1	А
	Through	4.2	0.1	А	4.4		А
	Right Turn	5.6		А	5.8		А
Clergate Road	Left Turn	5.6		А	5.6		А
	Through	0.0	0.0	А	0.0	0.0	А
	Right Turn	5.5		Α	5.5		A

Pearce Lane West	Left Turn	5.6		А	5.7		А
	Through	0.0	0.4	А	0.2	0.9	А
	Right Turn	5.5		А	5.6		А

The results of the SIDRA analysis for the PM peak are similar to the AM peak and indicate the following:

- The intersection is expected to continue to operate with minimal queue lengths on all legs of the intersection;
- The overall average delay at the intersection increases by 1.4 seconds; and
- The intersection is expected to continue to operate with a good level of service.

Overall, the increase in traffic generated by the subdivision is expected to have a negligible impact to the operation of the intersection of Pearce Lane, Clergate Road which is expected to continue to operate with a good level of service.

4.4 Railway Level Crossings

Amber has contacted John Holland's Operational Team who operate the railway line that runs along the western boundary of the site. There response is provided below:

"...there are no pathed trains through the location in the morning. The up direction XPT ex Dubbo is normally through Orange between 1700 and 1800. Freight services generally get through Orange after 1800. This might average one per day. So a total of two per day in the evening."

There are two level crossings that would be traversed by development traffic which includes the level crossing at the Clergate Road site access and the level crossing on Pearce Lane near the northern access.

Given the infrequent number of services along the railway line and the low traffic volumes at each of the level crossings, the interaction of site traffic with the level crossings is not expected to create any congestion or safety impacts.

4.5 Summary

The SIDRA analysis provided a review of the impacts of the site traffic on the surrounding road network for the year 2028 based on the traffic volumes provided within the Orange Strategic Traffic Model and assumed the site had been fully constructed. The analysis is summarised in Table 15.

		AM Peak		PM Peak			
Intersection	Average Delay (sec)	95% Queue (m)	Level of Service	Average Delay (sec)	95% Queue (m)	Level of Service	
Leeds Parade / Northern Distributor Road	9.1	39.4	А	8.1	20.5	А	
Clergate Road / Northern Distributor Road	220.4	921.8	-	123.4	613.4	-	

Table 15: SIDRA Analysis Results Summary

Clergate Road / Farrell Road	3.9	13.5	-	3.2	18.0	-
Farrell Road / Telopea Way	13.0	14.5	В	14.7	26.1	В
Telopea Way / Northern Distributor Road	15.9	63.9	В	14.1	42.6	В
Clergate Road / Site Access (middle access)	4.5	2.7	-	4.5	2.4	-
Pearce Lane / Clergate Road / Site Access (northern access)	4.5	0.9	-	3.6	0.9	-

The analysis demonstrates that the surrounding road network is expected to continue to operate with good to satisfactory levels of service and acceptable queue lengths and delays with the exception of right turn movement from Clergate Road to Northern Distributor Road. However, it is noted that the right turn movement is already operating above capacity with the 2028 Orange Strategic Model traffic volumes. It is considered that this is due to the strategic model not accounting for the long delays and the associated redistribution of traffic on the road network as drivers choose a more suitable travel route.

Site traffic is able to avoid this right turn manoeuvre by either travelling eastbound on Northern Distributor Road via Leeds Parade and utilising the internal road network, or by utilising Farrell Road and Telopea Way. Each of these intersections has capacity to accommodate an increase in traffic generated by the redistribution of traffic.

Overall, it is concluded that site traffic will have a minor impact on the surrounding road network and the traffic volumes can be accommodated on the road network in a safe and efficient manner.

5. Access and Internal Road Layout

A master plan for the site has been prepared by Premise which has previously been shown within Figure 7 and identifies the locations of the proposed accesses. The proposed road layout is considered to provide suitable vehicle circulation within the site. It is recommended that the internal road layout and cross-sections be established in accordance with the Orange Development Control Plan and relevant design standards.

The internal and external intersections are recommended to be designed to meet the Safe Intersection Sight Distance specified within *Austroads Guide to Road Design – Part 4A: Unsignalised Intersections* to ensure the sight distance at each intersection allows the safe movement of vehicles on the road network.

The traffic assessment indicates that the new accesses via Clergate Road and Pearce Lane are expected to operate with minimal delays and queue lengths. Notwithstanding this, it is recommended that any future design for the accesses consider the provision of turn facilities in accordance with the relevant Austroads Guidelines.

It is recommended that the Clergate Road access design give careful consideration to the operation of the Western Railway Line and associated level crossing upgrade given the proximity of the railway line to the intersection with Clergate Road.



6. Car Parking

The applicant has advised that all car parking is to be provided in accordance with the Development Control Plan and relevant Council design documents. In addition, the carriageway width of the internal road network is expected to allow for two-way traffic and on-street parallel parking once fully constructed. The on-street spaces will be available to service the needs of visitors of future residents within the subdivision. Accordingly, the subdivision is not expected to generate any parking impacts and the parking demand can be readily accommodated internally within the site.



7. Alternative Transport Modes

It is recommended that as part of any future development of the site that TfNSW be contacted to determine whether there will be any future bus services provided within the site. The internal road widths and intersections will need to be designed to accommodate the relevant vehicle and provisions will need to be made to allowed for bus stop facilities.

It is also recommended that consideration be given to provide a shared path that connects with the existing path along Leeds Parade to provide a pedestrian / cyclist connection between the site and the Orange CBD.



8. Conclusions

Amber has reviewed the traffic and parking matters of the proposed amendment to the LEP which would reduce the minimum lot size on-site and increase the yield of the land from 450 lots to approximately 700 lots. The proposal also introduces an upgrade of a private level crossing to a public level crossing to provide access via Clergate Road and introduces a third access via Pearce Lane.

Based on the above assessment, it is concluded that:

- The development is expected to generate approximately 5,180 vehicle movements per day, and 546 and 497 vehicle movements (two-way total) in the morning and evening peak hours respectively;
- Site traffic will have a minor impact on the surrounding road network, with modest increases to queue lengths and delays, and the traffic volumes can be accommodated on the road network in a safe and efficient manner;
- The access locations allow traffic to be distributed on the road network and they are not expected to create any operational or safety issues at the nearby railway level crossings;
- Car parking for the individual lots is to be provided in accordance with the DCP, with onstreet parking provided for visitors; and
- It is recommended that future consideration be given to providing sustainable transport facilities within the site that link with existing bus routes and shared paths.

Therefore, it is concluded that the traffic and parking aspects of the proposed development are satisfactory, and the development will have a minimal impact on the surrounding environment.



Appendix A

Orange Traffic Model Outputs



Intersection	Approach	Approach Movement	Movement	2018 AM	2018 PM	2028 AM	2028 PM	Comments						
			NBL	113	199	183	232							
	Leeds Parade	NB	NBT	52	21	57	23	-						
			NBR2	91	20	116	22		114					
			NBU	0	0	0	0		179 0 0 0 0 0 0 0 0 0 0					
			SBL2	4	1	5	4		2412 · · · · · · · · · · · · · · · · · · ·					
	Leeds Parade	SB	SB	SBL	27	43	30	39 65		0				
			SBR	64	153	69	161		P 7					
			SBU	0	0	0	0	For TAZ 316 the trips coming form and going to south						
			WBL2	0	0	0	0	Northern Distributor Road (NDR) are loaded from						
Leeds Parade / Northern Distributor	Hanrahan Place	WB	WBT	19	63	34	73	Hanrahan Place (and vice-a-versa) are zero. In other						
			WBR	4	3	7	4	words, NWBR2 and WBL2 movements have zero	Course of the second seco					
			WBU	0	0	0	0	volume. Hanrahan Place is another loading point of TAZ						
			NWBL	24	64	27	55	316 trips.						
	Northern Distributor Road	NWB	NWBR	36	12	31	13							
			NWBR2	0	0	0	0							
			NWBU	0	0	0	0							
			SEBL2	43	15	48	17							
	Northern Distributor Road	SEB	SEBL	356	217	412	252							
		-	SEBR	190	217	244	250							
			SEBU	0	0	0	0		0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0					
	Clergate Road	SB	SBL	262	173	301	220		- 4					
	5		-	0		SBR FBI	65	195	91	192		340		
Clergate Road / Northern Distributor	Northern Distributor Road	Northern Distributor Road	Northern Distributor Road	Northern Distributor Road	Northern Distributor Road	Northern Distributor Road	EB	EBT	464	334	566	388		
	Northern Distributor Road	WB	WBT	252	536	311	598		Leeds 200					
	Northern Distributor Rodu		WBR	184	240	266	273							
	Clergate Road	NB	NBL	0	17	0	16	The trip attractions for the zone adjacent to Clergate						
			SBT	220	361	363	400	the zone is getting loaded on south of Northern						
Clergate Road / Farrell Road	Clergate Road	SB	SBR	17	61	22	149	Distributor road (Anson street) instead from Clergate	1//					
	Farrell Road	EB	EBL	151	39	214	36	road. As a result NBL volume is zero at this intersection						
			EBR	36	5	120	215	in AM Peak.						
	Telopea Way	NB	NBR	135	144	165	117							
Farrell Road / Telonea Way	Telonea Way	SB	SBL	22	38	29	45							
		36	SBR	126	265	136	233		Contraction of the second s					
	Farrell Road	WB	WBL	168	71	20	116 42							
			SBL	154	161	162	195							
	Telopea Way (Farrell Road)	SB	SBR	139	176	127	155							
Telopea Way (Farrell Road) / Northern	Northern Distributor Road	EB	EBL	160	214	223	235							
Distributor Road			EBT	341	238	547	325							
	Northern Distributor Road	WB	WBR	67	188	81	198							
	Clargata Boad	CD.	SBL	0	0	0	0							
		зв	SBR	0	1	0	1	The OD demand for the TAZ connected through the SB						
Pearce Lane / Clergate Road	Pearce Lane	EB	EBL	2	0	2	0	approach of this intersection is very low (almost zero)						
			WBT	12	3	15	4	as a results SB approach has zero/one volume.						
	Pearce Lane	WB	WBR	0	0	0	0							
	Clergate Road	NB	NBL	0	0	0	0							
			NBT	3	10	3	12	No growth predicted in TAZs on left side of the						
Dawson Gates Road / Clergate Road	Clergate Road	SB	SBT	10	2	12	3	intersection and therefore NBL and EBR movements						
			EBL	4	1	4	1	shows zero volume.						
	Dawson Gates Road	ÉB	EBR	0	0	0	0							
	Burrendong Wav	NB	NBT	47	79	116	73							
Dawson Catos Dand / Burnardan			NBR	2	0	8	0	Growth predicted in the TAZs on right side of this						
Dawson Gates Road / Burrendong Wav	Burrendong Way	SB	SBL	4/	23 46	48	23 56	intersection is very low, as a results NBR movments has						
, way	Deuroen Catal Barad	14/5	WBL	1	3	1	10	zero volume in PM peak and negligible in AM peak.						
	Dawson Gates Road	WB	WBR	14	37	39	32							




Appendix B

Masterplan





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S ESTATE PTY LTD	JOB CODE	
DIVISION	221025	_02
	SHEET NUMBER	REV
	TP01	Е

Appendix C

Guidelines for Assessing Intersection Performance



The *RTA Guide to Traffic Generating Developments* (October 2002, Issue 2.2), details the assessment of intersections. The assessment of the level of service of an intersection is based on the evaluation of the following Measures of Effectiveness:

- Average delay (seconds/veh) (all forms of control)
- Delay to critical movement (seconds/veh) (all forms of control)
- Degree of saturation (traffic signals and roundabouts)
- Cycle length (traffic signals)

SIDRA was used to calculate the relevant intersection parameters. The SIDRA software is an advanced lane-based micro-analytical tool for design and evaluation of individual intersections and networks of intersections including modelling of separate movement classes (light vehicles, heavy vehicles, buses, cyclists, large trucks, light rail / trams and so on). It provides estimates of capacity, level of service and a wide range of performance measures, including; delay, queue length and stops for vehicles and pedestrians, as well as fuel consumption, pollution emissions and operating costs.

It can be used to analyse signalised intersections (fixed-time / pretimed and actuated), signalised and unsignalised pedestrian crossings, roundabouts (unsignalised), roundabouts with metering signals, fully-signalised roundabouts, two-way stop sign and give-way / yield sign control, all-way stop sign control, single point interchanges (signalised), freeway diamond interchanges (signalised, roundabout, sign control), diverging diamond interchanges and other alternative intersections and interchanges. It can also be used for uninterrupted traffic flow conditions and merge analysis.

The best indicator of the level of service at an intersection is the average delay experienced by vehicles at that intersection. For traffic signals, the average delay over all movements should be taken. For roundabouts and priority control intersections (with Stop and Give Way signs or operating under the T-junction rule) the critical movement for level of service assessment should be that with the highest average delay.

With traffic signals, delays per approach tend to be equalised, subject to any over-riding requirements of signal co-ordination as well as to variations within individual movements. With roundabouts and priority - control intersections, the critical criterion for assessment is the movement with the highest delay per vehicle. With this type of control the volume balance might be such that some movements suffer high levels of delay while other movements have minimal delay. An overall average delay for the intersection of 25 seconds might not be satisfactory if the average delay on one movement is 60 seconds.

The average delay for level of service E should be no more than 70 seconds. The accepted maximum practical cycle length for traffic signals under saturated conditions is 120 - 140 seconds. Under these conditions 120 seconds is near maximum for two and three phase intersections and 140 seconds near maximum for more complex phase designs. Drivers and pedestrians expect cycle lengths of these magnitudes and their inherent delays in peak hours. A cycle length of 140 seconds for an intersection which is almost saturated has an average vehicle delay of about 70 seconds, although this can vary. If the average vehicle delay is more than 70 seconds, the intersection is assumed to be at Level of Service F.

Table 16 sets out average delays for different levels of service. There is no consistent correlation between definitions of levels of service for road links as defined elsewhere in this section, and the ranges set out in Table 16. In assigning a level of service, the average delay to the motoring public needs to be considered, keeping in mind the location of the intersection. For example, drivers in inner urban areas of Sydney have a higher tolerance of delay than drivers in country areas. Table 16 provides a recommended baseline for assessment.



Level of Service	Average Delay per Vehicle (sec/veh)	Traffic Signals, Roundabouts	Give Way and Stop Signs
А	Less than 14	Good operation	Good operation
В	15 – 28	Good with acceptable delays and spare capacity	Acceptable delays and spare capacity
С	29 - 42	Satisfactory	Satisfactory, but accident study required
D	43 - 56	Operating near capacity	Near capacity and accident study required
E	57 - 70	At capacity Signals, incidents will cause excessive delays Roundabouts require other control mode	At capacity, require other control mode

Table 16: Level of Service Criteria for Intersections

The figures in Table 16 are intended as a guide only. Any particular assessment should take into account site-specific factors including maximum queue lengths (and their effect on lane blocking), the influence of nearby intersections and the sensitivity of the location to delays. In many situations, a comparison of the current and future average delay provides a better appreciation of the impact of a proposal, and not simply the change in the level of service.



Appendix D

SIDRA Results



SITE LAYOUT V Site: 101 [Northern Distributor Road / Leeds Parade (Site Folder: General)]

AM Peak - Strategic Model Volumes 2028 Site Category: (None) Roundabout

Layout pictures are schematic functional drawings reflecting input data. They are not design drawings.



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V Site: 101 [Northern Distributor Road / Leeds Parade (Site Folder: General)]

AM Peak - Strategic Model Volumes 2028 Site Category: (None) Roundabout

Vehicle Movement Performance														
Mov	Turn	INP	UT	DEM	AND	Deg.	Aver.	Level of	95% BA	CK OF	Prop.	Effective	Aver.	Aver.
ID		Total	MES HV L	FLO	WS HV L	Satn	Delay	Service	QUE	Dist L	Que	Stop Rate	No. Cvcles	Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m		Hate	0,000	km/h
South	n: Leed	ds Parade	e South											
1	L2	183	5.0	193	5.0	0.200	5.6	LOS A	0.9	6.4	0.49	0.63	0.49	54.4
2	T1	57	5.0	60	5.0	0.239	4.9	LOS A	1.1	8.1	0.48	0.69	0.48	53.0
3a	R1	86	5.0	91	5.0	0.239	9.4	LOS A	1.1	8.1	0.48	0.69	0.48	52.6
3b	R3	116	5.0	122	5.0	0.239	11.8	LOS B	1.1	8.1	0.48	0.69	0.48	54.0
Appro	oach	442	5.0	465	5.0	0.239	7.9	LOS A	1.1	8.1	0.48	0.66	0.48	53.8
South	nEast:	Northern	Distribu	tor Road	East									
21b	L3	27	5.0	28	5.0	0.019	4.7	LOS A	0.1	0.6	0.36	0.50	0.36	54.5
21a	L1	290	5.0	305	5.0	0.217	4.6	LOS A	1.2	8.6	0.49	0.48	0.49	55.7
23a	R1	31	5.0	33	5.0	0.034	9.5	LOS A	0.1	1.1	0.46	0.64	0.46	52.6
23	R2	1	5.0	1	5.0	0.034	10.7	LOS B	0.1	1.1	0.46	0.64	0.46	53.3
Appro	oach	349	5.0	367	5.0	0.217	5.1	LOS A	1.2	8.6	0.47	0.50	0.47	55.3
North	East:	Hanrahai	n Place											
24	L2	1	5.0	1	5.0	0.128	9.4	LOS A	0.7	5.1	0.74	0.82	0.74	51.0
24a	L1	32	5.0	34	5.0	0.128	8.5	LOS A	0.7	5.1	0.74	0.82	0.74	52.0
26a	R1	34	5.0	36	5.0	0.128	12.9	LOS B	0.7	5.1	0.74	0.82	0.74	52.1
26b	R3	7	5.0	7	5.0	0.128	15.3	LOS B	0.7	5.1	0.74	0.82	0.74	53.4
Appro	oach	74	5.0	78	5.0	0.128	11.2	LOS B	0.7	5.1	0.74	0.82	0.74	52.2
North	n: Leed	ls Parade	North											
7b	L3	4	5.0	4	5.0	0.055	7.5	LOS A	0.2	1.7	0.65	0.72	0.65	52.7
7a	L1	15	5.0	16	5.0	0.055	8.1	LOS A	0.2	1.7	0.65	0.72	0.65	54.6
8	T1	30	5.0	32	5.0	0.108	6.9	LOS A	0.5	3.8	0.65	0.75	0.65	53.5
9	R2	69	5.0	73	5.0	0.108	11.9	LOS B	0.5	3.8	0.66	0.78	0.66	52.7
Appro	oach	118	5.0	124	5.0	0.108	10.0	LOS A	0.5	3.8	0.65	0.76	0.65	53.1
West	: North	nern Distr	ibutor R	oad West										
10	L2	17	5.0	18	5.0	0.190	5.9	LOS A	0.9	6.4	0.49	0.57	0.49	54.4
10a	L1	119	5.0	125	5.0	0.190	5.4	LOS A	0.9	6.4	0.49	0.57	0.49	55.7
12a	R1	412	5.0	434	5.0	0.565	9.9	LOS A	4.2	30.9	0.61	0.72	0.62	51.8
12	R2	244	5.0	257	5.0	0.565	11.1	LOS B	4.2	30.9	0.61	0.72	0.62	52.4
Appro	oach	792	5.0	834	5.0	0.565	9.5	LOS A	4.2	30.9	0.59	0.69	0.60	52.6
All Vehic	les	1775	5.0	1868	5.0	0.565	8.3	LOS A	4.2	30.9	0.55	0.66	0.55	53.4

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Roundabout LOS Method: SIDRA Roundabout LOS.

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Geometric Delay is included).

Queue Model: SIDRA Standard.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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V Site: 101 [Northern Distributor Road / Leeds Parade (Site Folder: General)]

AM Peak - Future Volumes 2028 Site Category: (None) Roundabout

Vehi	Vehicle Movement Performance													
Mov	Turn	INP	UT	DEM.	AND	Deg.	Aver.	Level of	95% BA	CK OF	Prop.	Effective	Aver.	Aver.
ID		VOLU	IMES	FLO	WS HV 1	Satn	Delay	Service	QUE [\/eh	EUE Dist 1	Que	Stop Rate	No. Cycles	Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m		nate	Cycles	km/h
South	n: Lee	ds Parade	e South											
1	L2	187	5.0	197	5.0	0.222	6.2	LOS A	1.0	7.0	0.53	0.68	0.53	54.2
2	T1	90	5.0	95	5.0	0.282	5.2	LOS A	1.3	9.6	0.53	0.71	0.53	53.2
3a	R1	86	5.0	91	5.0	0.282	9.7	LOS A	1.3	9.6	0.53	0.71	0.53	52.8
3b	R3	116	5.0	122	5.0	0.282	12.1	LOS B	1.3	9.6	0.53	0.71	0.53	54.1
Appro	oach	479	5.0	504	5.0	0.282	8.1	LOS A	1.3	9.6	0.53	0.70	0.53	53.8
South	nEast:	Northern	Distribu	itor Road	East									
21b	L3	27	5.0	28	5.0	0.020	5.3	LOS A	0.1	0.7	0.43	0.53	0.43	54.3
21a	L1	294	5.0	309	5.0	0.247	5.7	LOS A	1.5	10.7	0.62	0.57	0.62	55.0
23a	R1	41	5.0	43	5.0	0.049	10.4	LOS B	0.2	1.7	0.57	0.70	0.57	52.2
23	R2	1	5.0	1	5.0	0.049	11.6	LOS B	0.2	1.7	0.57	0.70	0.57	52.9
Appro	oach	363	5.0	382	5.0	0.247	6.2	LOS A	1.5	10.7	0.60	0.58	0.60	54.6
North	East:	Hanrahai	n Place											
24	L2	1	5.0	1	5.0	0.157	11.9	LOS B	0.9	6.3	0.81	0.90	0.81	49.4
24a	L1	32	5.0	34	5.0	0.157	11.2	LOS B	0.9	6.3	0.81	0.90	0.81	50.3
26a	R1	34	5.0	36	5.0	0.157	15.3	LOS B	0.9	6.3	0.81	0.90	0.81	50.4
26b	R3	7	5.0	7	5.0	0.157	17.7	LOS B	0.9	6.3	0.81	0.90	0.81	51.7
Appro	oach	74	5.0	78	5.0	0.157	13.7	LOS B	0.9	6.3	0.81	0.90	0.81	50.5
North	n: Leec	ls Parade	e North											
7b	L3	4	5.0	4	5.0	0.185	8.0	LOS A	0.8	6.1	0.70	0.80	0.70	52.3
7a	L1	54	5.0	57	5.0	0.185	8.8	LOS A	0.8	6.1	0.70	0.80	0.70	54.3
8	T1	161	5.0	169	5.0	0.362	7.4	LOS A	2.0	14.9	0.74	0.82	0.76	53.1
9	R2	161	5.0	169	5.0	0.362	12.7	LOS B	2.0	14.9	0.76	0.83	0.78	52.8
Appro	oach	380	5.0	400	5.0	0.362	9.8	LOS A	2.0	14.9	0.74	0.82	0.76	53.2
West	: North	nern Distr	ibutor Re	oad West										
10	L2	40	5.0	42	5.0	0.231	6.2	LOS A	1.1	8.1	0.53	0.62	0.53	54.2
10a	L1	119	5.0	125	5.0	0.231	5.8	LOS A	1.1	8.1	0.53	0.62	0.53	55.5
12a	R1	428	5.0	451	5.0	0.615	10.8	LOS B	5.4	39.4	0.69	0.78	0.75	51.5
12	R2	260	5.0	274	5.0	0.615	12.0	LOS B	5.4	39.4	0.69	0.78	0.75	52.2
Appro	oach	847	5.0	892	5.0	0.615	10.2	LOS B	5.4	39.4	0.66	0.75	0.71	52.4
All Vehic	les	2143	5.0	2256	5.0	0.615	9.1	LOS A	5.4	39.4	0.64	0.73	0.66	53.1

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Roundabout LOS Method: SIDRA Roundabout LOS.

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Geometric Delay is included).

Queue Model: SIDRA Standard.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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W Site: 101 [Northern Distributor Road / Leeds Parade (Site Folder: General)]

PM Peak - Strategic Model Volumes 2028 Site Category: (None) Roundabout

Vehi	Vehicle Movement Performance													
Mov	Turn	INP	UT	DEM	AND	Deg.	Aver.	Level of	95% BA	CK OF	Prop.	Effective	Aver.	Aver.
TID		VOLU	MES HV.1_	FLO	WS HV1_	Satn	Delay	Service	QUE	UE Dist 1_	Que	Stop Rate	No. Cycles	Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m		Tate	Cycles	km/h
South	n: Leed	ds Parade	e South											
1	L2	232	5.0	244	5.0	0.239	6.3	LOS A	1.1	8.2	0.57	0.66	0.57	54.1
2	T1	23	5.0	24	5.0	0.143	6.1	LOS A	0.6	4.3	0.57	0.76	0.57	53.0
3a	R1	58	5.0	61	5.0	0.143	10.6	LOS B	0.6	4.3	0.57	0.76	0.57	52.6
3b	R3	22	5.0	23	5.0	0.143	13.0	LOS B	0.6	4.3	0.57	0.76	0.57	53.9
Appro	bach	335	5.0	353	5.0	0.239	7.5	LOS A	1.1	8.2	0.57	0.69	0.57	53.7
South	nEast:	Northern	Distribu	tor Road	East									
21b	L3	55	5.0	58	5.0	0.039	4.9	LOS A	0.2	1.2	0.36	0.53	0.36	54.5
21a	L1	404	5.0	425	5.0	0.322	5.9	LOS A	1.9	13.8	0.60	0.57	0.60	55.1
23a	R1	13	5.0	14	5.0	0.015	10.1	LOS B	0.1	0.5	0.52	0.64	0.52	52.4
23	R2	1	5.0	1	5.0	0.015	11.3	LOS B	0.1	0.5	0.52	0.64	0.52	53.0
Appro	bach	473	5.0	498	5.0	0.322	5.9	LOS A	1.9	13.8	0.57	0.57	0.57	55.0
North	East:	Hanrahar	n Place											
24	L2	1	5.0	1	5.0	0.178	7.7	LOS A	0.8	6.2	0.64	0.79	0.64	51.7
24a	L1	48	5.0	51	5.0	0.178	7.5	LOS A	0.8	6.2	0.64	0.79	0.64	52.7
26a	R1	73	5.0	77	5.0	0.178	11.8	LOS B	0.8	6.2	0.64	0.79	0.64	52.8
26b	R3	4	5.0	4	5.0	0.178	14.2	LOS B	0.8	6.2	0.64	0.79	0.64	54.1
Appro	bach	126	5.0	133	5.0	0.178	10.2	LOS B	0.8	6.2	0.64	0.79	0.64	52.8
North	: Leed	ls Parade	North											
7b	L3	5	5.0	5	5.0	0.104	6.7	LOS A	0.4	3.1	0.56	0.66	0.56	53.1
7a	L1	39	5.0	41	5.0	0.104	6.7	LOS A	0.4	3.1	0.56	0.66	0.56	55.1
8	T1	65	5.0	68	5.0	0.204	6.3	LOS A	0.9	6.9	0.56	0.71	0.56	53.9
9	R2	161	5.0	169	5.0	0.204	11.3	LOS B	0.9	6.9	0.56	0.75	0.56	53.0
Appro	oach	270	5.0	284	5.0	0.204	9.4	LOS A	0.9	6.9	0.56	0.73	0.56	53.5
West	: North	nern Distr	ibutor Re	oad West										
10	L2	48	5.0	51	5.0	0.167	4.5	LOS A	0.8	5.7	0.31	0.44	0.31	55.2
10a	L1	102	5.0	107	5.0	0.167	4.0	LOS A	0.8	5.7	0.31	0.44	0.31	56.5
12a	R1	252	5.0	265	5.0	0.373	8.6	LOS A	2.3	16.9	0.33	0.60	0.33	52.7
12	R2	250	5.0	263	5.0	0.373	9.8	LOS A	2.3	16.9	0.33	0.60	0.33	53.3
Appro	bach	652	5.0	686	5.0	0.373	8.0	LOS A	2.3	16.9	0.33	0.57	0.33	53.7
All Vehic	les	1856	5.0	1954	5.0	0.373	7.7	LOS A	2.3	16.9	0.49	0.63	0.49	53.9

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Roundabout LOS Method: SIDRA Roundabout LOS.

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Geometric Delay is included).

Queue Model: SIDRA Standard.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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V Site: 101 [Northern Distributor Road / Leeds Parade (Site Folder: General)]

PM Peak - Future Volumes 2028 Site Category: (None) Roundabout

Vehicle Movement Performance														
Mov	Turn	INP	UT	DEM	AND	Deg.	Aver.	Level of	95% BA	CK OF	Prop.	Effective	Aver.	Aver.
ID		VOLU [Total	IMES HV 1	FLO [Total	WS HV 1	Satn	Delay	Service	QUE [Veh	:UE Dist 1	Que	Stop Rate	No. Cvcles	Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m		- Tiato		km/h
South	n: Leed	ds Parade	e South											
1	L2	245	5.0	258	5.0	0.259	6.7	LOS A	1.2	8.9	0.60	0.68	0.60	54.0
2	T1	127	5.0	134	5.0	0.265	6.2	LOS A	1.2	8.7	0.61	0.71	0.61	53.9
3a	R1	58	5.0	61	5.0	0.265	10.7	LOS B	1.2	8.7	0.61	0.71	0.61	53.5
3b	R3	22	5.0	23	5.0	0.265	13.0	LOS B	1.2	8.7	0.61	0.71	0.61	54.9
Appro	bach	452	5.0	476	5.0	0.265	7.4	LOS A	1.2	8.9	0.61	0.69	0.61	54.0
South	nEast:	Northern	Distribu	tor Road	East									
21b	L3	55	5.0	58	5.0	0.040	5.1	LOS A	0.2	1.3	0.40	0.54	0.40	54.4
21a	L1	417	5.0	439	5.0	0.349	6.5	LOS A	2.2	15.9	0.66	0.61	0.66	54.9
23a	R1	44	5.0	46	5.0	0.052	10.5	LOS B	0.2	1.8	0.57	0.70	0.57	52.2
23	R2	1	5.0	1	5.0	0.052	11.7	LOS B	0.2	1.8	0.57	0.70	0.57	52.9
Appro	bach	517	5.0	544	5.0	0.349	6.7	LOS A	2.2	15.9	0.62	0.61	0.62	54.6
North	East:	Hanrahai	n Place											
24	L2	1	5.0	1	5.0	0.194	8.3	LOS A	1.0	7.0	0.69	0.83	0.69	51.3
24a	L1	48	5.0	51	5.0	0.194	8.2	LOS A	1.0	7.0	0.69	0.83	0.69	52.3
26a	R1	73	5.0	77	5.0	0.194	12.4	LOS B	1.0	7.0	0.69	0.83	0.69	52.4
26b	R3	4	5.0	4	5.0	0.194	14.8	LOS B	1.0	7.0	0.69	0.83	0.69	53.7
Appro	bach	126	5.0	133	5.0	0.194	10.8	LOS B	1.0	7.0	0.69	0.83	0.69	52.4
North	: Leed	ls Parade	e North											
7b	L3	5	5.0	5	5.0	0.143	6.8	LOS A	0.6	4.5	0.59	0.68	0.59	52.9
7a	L1	52	5.0	55	5.0	0.143	6.8	LOS A	0.6	4.5	0.59	0.68	0.59	54.9
8	T1	110	5.0	116	5.0	0.280	6.4	LOS A	1.4	10.4	0.61	0.73	0.61	53.7
9	R2	192	5.0	202	5.0	0.280	11.5	LOS B	1.4	10.4	0.62	0.76	0.62	53.1
Appro	oach	359	5.0	378	5.0	0.280	9.2	LOS A	1.4	10.4	0.61	0.74	0.61	53.5
West	: North	nern Distr	ibutor Re	oad West										
10	L2	121	5.0	127	5.0	0.248	5.2	LOS A	1.3	9.3	0.46	0.55	0.46	54.5
10a	L1	102	5.0	107	5.0	0.248	4.7	LOS A	1.3	9.3	0.46	0.55	0.46	55.8
12a	R1	257	5.0	271	5.0	0.431	9.3	LOS A	2.8	20.5	0.51	0.67	0.51	52.1
12	R2	255	5.0	268	5.0	0.431	10.5	LOS B	2.8	20.5	0.51	0.67	0.51	52.7
Appro	bach	735	5.0	774	5.0	0.431	8.4	LOS A	2.8	20.5	0.50	0.64	0.50	53.2
All Vehic	les	2189	5.0	2304	5.0	0.431	8.1	LOS A	2.8	20.5	0.58	0.67	0.58	53.7

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Roundabout LOS Method: SIDRA Roundabout LOS.

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Geometric Delay is included).

Queue Model: SIDRA Standard.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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SITE LAYOUT V Site: 101 [Northern Distributor Road / Clergate Road (Site Folder: General)]

PM Peak - Future Volumes 2028 Site Category: (None) Give-Way (Two-Way)

Layout pictures are schematic functional drawings reflecting input data. They are not design drawings.



Northern Distrib... East

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V Site: 101 [Northern Distributor Road / Clergate Road (Site Folder: General)]

AM Peak - Strategic Model Volumes 2028 Site Category: (None) Give-Way (Two-Way)

Vehi	Vehicle Movement Performance													
Mov	Turn	INP	TUT	DEM	AND	Deg.	Aver.	Level of	95% B/	ACK OF	Prop.	Effective	Aver.	Aver.
ID		VOLL	JMES	FLO	WS	Satn	Delay	Service	QU	EUE	Que	Stop	No.	Speed
		[Total	HV]	[Total	HV]				[Veh.	Dist]		Rate	Cycles	
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
East:	North	ern Distri	butor Ro	ad East										
5	T1	311	3.0	327	3.0	0.172	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	59.9
6	R2	266	3.0	280	3.0	0.368	10.9	LOS B	1.9	13.9	0.68	0.93	0.88	49.4
Appro	bach	577	3.0	607	3.0	0.368	5.0	NA	1.9	13.9	0.31	0.43	0.40	54.6
North	: Cler	gate Roa	d											
7	L2	91	3.0	96	3.0	0.113	8.4	LOS A	0.4	3.0	0.54	0.76	0.54	51.4
9	R2	301	3.0	317	3.0	1.754	710.9	LOS F	92.6	664.5	1.00	5.11	17.35	4.6
Appro	oach	392	3.0	413	3.0	1.754	547.8	LOS F	92.6	664.5	0.89	4.10	13.45	5.9
West	: North	nern Distr	ibutor Ro	oad West										
10	L2	118	3.0	124	3.0	0.380	5.7	LOS A	0.0	0.0	0.00	0.10	0.00	57.1
11	T1	566	3.0	596	3.0	0.380	0.1	LOS A	0.0	0.0	0.00	0.10	0.00	58.8
Appro	oach	684	3.0	720	3.0	0.380	1.1	NA	0.0	0.0	0.00	0.10	0.00	58.5
All Vehic	les	1653	3.0	1740	3.0	1.754	132.1	NA	92.6	664.5	0.32	1.16	3.33	18.6

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

Delay Model: SIDRA Standard (Geometric Delay is included).

Queue Model: SIDRA Standard.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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V Site: 101 [Northern Distributor Road / Clergate Road (Site Folder: General)]

AM Peak - Future Volumes 2028 Site Category: (None) Give-Way (Two-Way)

Vehi	Vehicle Movement Performance														
Mov	Turn	INP	UT	DEM/	AND	Deg.	Aver.	Level of	95% BA	ACK OF	Prop.	Effective	Aver.	Aver.	
ID		VOLU	IMES	FLO'	WS	Satn	Delay	Service	QUI	EUE	Que	Stop	No.	Speed	
		[Total	HV]	[Total	HV]				[Veh.	Dist]		Rate	Cycles	1	
		ven/n	%	ven/n	%	V/C	sec		ven	m				Km/n	
East:	North	ern Distri	butor Ro	ad East											
5	T1	403	3.0	424	3.0	0.223	0.1	LOS A	0.0	0.0	0.00	0.00	0.00	59.9	
6	R2	274	3.0	288	3.0	0.403	11.6	LOS B	2.2	15.5	0.71	0.96	0.97	48.9	
Appro	bach	677	3.0	713	3.0	0.403	4.7	NA	2.2	15.5	0.29	0.39	0.39	54.9	
North	. Clerr	nate Roa	Ч												
			u 												
7	L2	123	3.0	129	3.0	0.158	8.7	LOS A	0.6	4.3	0.56	0.79	0.56	51.2	
9	R2	315	3.0	332	3.0	2.374	1266.5	LOS F	128.4	921.8	1.00	5.38	18.97	2.7	
Appro	bach	438	3.0	461	3.0	2.374	913.3	LOS F	128.4	921.8	0.88	4.09	13.80	3.7	
West	: North	nern Distr	ibutor Ro	oad West											
10	L2	129	3.0	136	3.0	0.399	5.7	LOS A	0.0	0.0	0.00	0.11	0.00	57.1	
11	T1	589	3.0	620	3.0	0.399	0.1	LOS A	0.0	0.0	0.00	0.11	0.00	58.8	
Appro	oach	718	3.0	756	3.0	0.399	1.1	NA	0.0	0.0	0.00	0.11	0.00	58.5	
All Vehic	les	1833	3.0	1929	3.0	2.374	220.4	NA	128.4	921.8	0.32	1.16	3.44	12.8	

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

Delay Model: SIDRA Standard (Geometric Delay is included).

Queue Model: SIDRA Standard.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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V Site: 101 [Northern Distributor Road / Clergate Road (Site Folder: General)]

PM Peak - Strategic Model Volumes 2028 Site Category: (None) Give-Way (Two-Way)

Vehi	Vehicle Movement Performance														
Mov	Turn	INP	UT	DEMA	AND	Deg.	Aver.	Level of	95% BA	ACK OF	Prop.	Effective	Aver.	Aver.	
ID		VOLU	IMES	FLO	WS	Satn	Delay	Service	QUI	EUE	Que	Stop	No.	Speed	
		[Total	HV]	[Total	HV]				[Veh.	Dist]		Rate	Cycles		
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h	
East:	North	ern Distri	butor Ro	ad East											
5	T1	598	3.0	629	3.0	0.331	0.1	LOS A	0.0	0.0	0.00	0.00	0.00	59.8	
6	R2	273	3.0	287	3.0	0.279	8.2	LOS A	1.3	9.6	0.58	0.78	0.60	51.3	
Appro	bach	871	3.0	917	3.0	0.331	2.6	NA	1.3	9.6	0.18	0.24	0.19	56.8	
North	: Cler	gate Roa	d												
7	L2	192	3.0	202	3.0	0.187	7.3	LOS A	0.8	5.6	0.47	0.69	0.47	52.0	
9	R2	220	3.0	232	3.0	1.540	529.7	LOS F	57.3	411.5	1.00	3.93	12.94	6.0	
Appro	oach	412	3.0	434	3.0	1.540	286.3	LOS F	57.3	411.5	0.75	2.42	7.13	10.3	
West	: North	nern Distr	ibutor Ro	oad West											
10	L2	99	3.0	104	3.0	0.271	5.6	LOS A	0.0	0.0	0.00	0.12	0.00	57.1	
11	T1	388	3.0	408	3.0	0.271	0.1	LOS A	0.0	0.0	0.00	0.12	0.00	58.8	
Appro	oach	487	3.0	513	3.0	0.271	1.2	NA	0.0	0.0	0.00	0.12	0.00	58.4	
All Vehic	les	1770	3.0	1863	3.0	1.540	68.3	NA	57.3	411.5	0.26	0.72	1.75	27.8	

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

Delay Model: SIDRA Standard (Geometric Delay is included).

Queue Model: SIDRA Standard.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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V Site: 101 [Northern Distributor Road / Clergate Road (Site Folder: General)]

PM Peak - Future Volumes 2028 Site Category: (None) Give-Way (Two-Way)

Vehi	Vehicle Movement Performance														
Mov	Turn	INP	UT	DEM	AND	Deg.	Aver.	Level of	95% B/	ACK OF	Prop.	Effective	Aver.	Aver.	
ID		VOLU	IMES	FLO	WS	Satn	Delay	Service	QU	EUE	Que	Stop	No.	Speed	
		[Total	HV]	[Total	HV]				[Veh.	Dist]		Rate	Cycles	I	
		ven/n	%	ven/n	%	V/C	sec	_	ven	m	_	_	_	Km/n	
East:	North	ern Distri	butor Ro	ad East											
5	T1	629	3.0	662	3.0	0.349	0.1	LOS A	0.0	0.0	0.00	0.00	0.00	59.8	
6	R2	298	3.0	314	3.0	0.356	9.7	LOS A	1.9	13.9	0.65	0.91	0.80	50.2	
Appro	bach	927	3.0	976	3.0	0.356	3.2	NA	1.9	13.9	0.21	0.29	0.26	56.3	
North	: Cler	gate Road	b												
7	L2	203	3.0	214	3.0	0.217	7.9	LOS A	0.9	6.4	0.52	0.74	0.52	51.8	
9	R2	225	3.0	237	3.0	2.116	1044.7	LOS F	85.4	613.4	1.00	4.31	14.89	3.2	
Appro	oach	428	3.0	451	3.0	2.116	552.9	LOS F	85.4	613.4	0.77	2.62	8.08	5.8	
West	: North	nern Distr	ibutor Ro	oad West											
10	L2	133	3.0	140	3.0	0.331	5.7	LOS A	0.0	0.0	0.00	0.13	0.00	56.9	
11	T1	461	3.0	485	3.0	0.331	0.1	LOS A	0.0	0.0	0.00	0.13	0.00	58.6	
Appro	oach	594	3.0	625	3.0	0.331	1.4	NA	0.0	0.0	0.00	0.13	0.00	58.2	
All Vehic	les	1949	3.0	2052	3.0	2.116	123.4	NA	85.4	613.4	0.27	0.75	1.90	19.5	

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

Delay Model: SIDRA Standard (Geometric Delay is included).

Queue Model: SIDRA Standard.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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SITE LAYOUT

Site: 101 [Clergate Road / Farrell Road (Site Folder: General)]

AM Peak - Strategic Model Volumes 2028 Site Category: (None) Stop (Two-Way)

Layout pictures are schematic functional drawings reflecting input data. They are not design drawings.



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Site: 101 [Clergate Road / Farrell Road (Site Folder: General)]

AM Peak - Strategic Model Volumes 2028 Site Category: (None) Stop (Two-Way)

Vehi	Vehicle Movement Performance														
Mov	Turn	INP	UT	DEM	AND	Deg.	Aver.	Level of	95% BA	ACK OF	Prop.	Effective	Aver.	Aver.	
ID		VOLU	IMES	FLO	WS	Satn	Delay	Service	QUE	EUE	Que	Stop	No.	Speed	
		[Total	HV]	[Total	HV]				[Veh.	Dist]		Rate	Cycles	1 //	
		ven/h	%	ven/n	%	V/C	sec		ven	m				Km/h	
South	n: Cler	gate Roa	d South												
1	L2	30	3.0	32	3.0	0.229	5.6	LOS A	0.0	0.0	0.00	0.04	0.00	57.7	
2	T1	384	3.0	404	3.0	0.229	0.1	LOS A	0.0	0.0	0.00	0.04	0.00	59.5	
Appro	oach	414	3.0	436	3.0	0.229	0.5	NA	0.0	0.0	0.00	0.04	0.00	59.4	
North	n: Clerg	gate Roa	d North												
8	T1	363	3.0	382	3.0	0.221	0.2	LOS A	0.3	1.9	0.08	0.04	0.08	59.3	
9	R2	22	3.0	23	3.0	0.221	7.7	LOS A	0.3	1.9	0.08	0.04	0.08	57.0	
Appro	oach	385	3.0	405	3.0	0.221	0.6	NA	0.3	1.9	0.08	0.04	0.08	59.2	
West	: Farre	ell Road													
10	L2	214	3.0	225	3.0	0.320	10.9	LOS B	1.5	10.8	0.55	0.98	0.62	50.0	
12	R2	29	3.0	31	3.0	0.320	15.9	LOS C	1.5	10.8	0.55	0.98	0.62	49.6	
Appro	oach	243	3.0	256	3.0	0.320	11.5	LOS B	1.5	10.8	0.55	0.98	0.62	50.0	
All Vehic	les	1042	3.0	1097	3.0	0.320	3.1	NA	1.5	10.8	0.16	0.26	0.17	56.8	

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

Delay Model: SIDRA Standard (Geometric Delay is included).

Queue Model: SIDRA Standard.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Site: 101 [Clergate Road / Farrell Road (Site Folder: General)]

AM Peak - Future Volumes 2028 Site Category: (None) Stop (Two-Way)

Vehi	cle M	ovemen	t Perfoi	rmance										
Mov	Turn	INP	UT	DEM	AND	Deg.	Aver.	Level of	95% BA	ACK OF	Prop.	Effective	Aver.	Aver.
ID		VOLU	IMES	FLO	WS	Satn	Delay	Service	QUI	EUE	Que	Stop	No.	Speed
		l Iotai veh/h	HV J %	[IOtal veh/h	HV J %	v/c	sec		[Ven. veh	Dist j m		Rate	Cycles	km/h
South	n: Cler	gate Roa	d South	VOII/II		v/0	000		VOIT					KI10/11
1	L2	30	3.0	32	3.0	0.235	5.6	LOS A	0.0	0.0	0.00	0.04	0.00	57.7
2	T1	395	3.0	416	3.0	0.235	0.1	LOS A	0.0	0.0	0.00	0.04	0.00	59.5
Appro	oach	425	3.0	447	3.0	0.235	0.5	NA	0.0	0.0	0.00	0.04	0.00	59.4
North	n: Clere	gate Road	d North											
8	T1	408	3.0	429	3.0	0.327	1.0	LOS A	1.4	10.0	0.30	0.14	0.32	57.7
9	R2	106	3.0	112	3.0	0.327	8.1	LOS A	1.4	10.0	0.30	0.14	0.32	55.4
Appro	oach	514	3.0	541	3.0	0.327	2.4	NA	1.4	10.0	0.30	0.14	0.32	57.2
West	: Farre	ell Road												
10	L2	235	3.0	247	3.0	0.369	11.4	LOS B	1.9	13.5	0.57	1.02	0.71	49.5
12	R2	29	3.0	31	3.0	0.369	19.7	LOS C	1.9	13.5	0.57	1.02	0.71	49.1
Appro	oach	264	3.0	278	3.0	0.369	12.3	LOS B	1.9	13.5	0.57	1.02	0.71	49.5
All Vehic	les	1203	3.0	1266	3.0	0.369	3.9	NA	1.9	13.5	0.25	0.30	0.29	56.0

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

Delay Model: SIDRA Standard (Geometric Delay is included).

Queue Model: SIDRA Standard.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Site: 101 [Clergate Road / Farrell Road (Site Folder: General)]

PM Peak - Strategic Model Volumes 2028 Site Category: (None) Stop (Two-Way)

Vehi	cle M	ovemen	t Perfo	mance										
Mov	Turn	INP	TUT	DEM	AND	Deg.	Aver.	Level of	95% BA	ACK OF	Prop.	Effective	Aver.	Aver.
ID		VOLU	JMES	FLO	WS	Satn	Delay	Service	QUE	EUE	Que	Stop	No.	Speed
		[Total	HV]	[Total	HV]				[Veh.	Dist]		Rate	Cycles	lune /h
South	n: Clar	ven/n raate Roa	70 nd South	ven/n	70	V/C	Sec	_	ven	111	_	_	_	KIII/II
Jour		gate noa												
1	L2	16	3.0	17	3.0	0.205	5.6	LOS A	0.0	0.0	0.00	0.03	0.00	57.9
2	T1	356	3.0	375	3.0	0.205	0.1	LOS A	0.0	0.0	0.00	0.03	0.00	59.7
Appro	oach	372	3.0	392	3.0	0.205	0.3	NA	0.0	0.0	0.00	0.03	0.00	59.6
North	n: Cler	gate Roa	d North											
8	T1	400	3.0	421	3.0	0.354	1.1	LOS A	1.8	13.0	0.35	0.19	0.38	57.2
9	R2	149	3.0	157	3.0	0.354	7.8	LOS A	1.8	13.0	0.35	0.19	0.38	55.0
Appro	oach	549	3.0	578	3.0	0.354	2.9	NA	1.8	13.0	0.35	0.19	0.38	56.5
West	: Farre	ell Road												
10	L2	36	3.0	38	3.0	0.075	9.9	LOS A	0.3	1.9	0.49	0.91	0.49	49.9
12	R2	11	3.0	12	3.0	0.075	16.6	LOS C	0.3	1.9	0.49	0.91	0.49	49.5
Appro	oach	47	3.0	49	3.0	0.075	11.5	LOS B	0.3	1.9	0.49	0.91	0.49	49.8
All Vehic	les	968	3.0	1019	3.0	0.354	2.3	NA	1.8	13.0	0.22	0.16	0.24	57.3

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

Delay Model: SIDRA Standard (Geometric Delay is included).

Queue Model: SIDRA Standard.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Site: 101 [Clergate Road / Farrell Road (Site Folder: General)]

PM Peak - Future Volumes 2028 Site Category: (None) Stop (Two-Way)

Vehi	cle M	ovemen	t Perfor	mance										
Mov	Turn	INP	UT	DEM	AND	Deg.	Aver.	Level of	95% BA	ACK OF	Prop.	Effective	Aver.	Aver.
ID		VOLU	IMES	FLO	WS	Satn	Delay	Service	QUI	EUE	Que	Stop	No.	Speed
		l lotal	HV J %	[lotal	HV J %	vic	200		[Veh.	Dist J m		Rate	Cycles	km/h
South	n: Cler	gate Roa	d South	VCII/II	70	0/0	300	_	VCIT		_	_	_	K11/11
1	L2	16	3.0	17	3.0	0.225	5.6	LOS A	0.0	0.0	0.00	0.02	0.00	57.9
2	T1	392	3.0	413	3.0	0.225	0.1	LOS A	0.0	0.0	0.00	0.02	0.00	59.7
Appro	oach	408	3.0	429	3.0	0.225	0.3	NA	0.0	0.0	0.00	0.02	0.00	59.6
North	n: Clerg	gate Road	d North											
8	T1	415	3.0	437	3.0	0.396	1.6	LOS A	2.5	18.0	0.42	0.23	0.50	56.7
9	R2	178	3.0	187	3.0	0.396	8.3	LOS A	2.5	18.0	0.42	0.23	0.50	54.5
Appro	oach	593	3.0	624	3.0	0.396	3.6	NA	2.5	18.0	0.42	0.23	0.50	56.0
West	: Farre	ll Road												
10	L2	103	3.0	108	3.0	0.161	10.4	LOS B	0.6	4.3	0.51	0.93	0.51	50.2
12	R2	11	3.0	12	3.0	0.161	19.4	LOS C	0.6	4.3	0.51	0.93	0.51	49.7
Appro	oach	114	3.0	120	3.0	0.161	11.2	LOS B	0.6	4.3	0.51	0.93	0.51	50.1
All Vehic	les	1115	3.0	1174	3.0	0.396	3.2	NA	2.5	18.0	0.27	0.22	0.32	56.6

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

Delay Model: SIDRA Standard (Geometric Delay is included).

Queue Model: SIDRA Standard.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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NETWORK LAYOUT

■ Network: N101 [Telopea Way / Farrell Road / Northern Distributor Road (Network Folder: General)]

New Network Network Category: (None)

Layout pictures are schematic functional drawings reflecting input data. They are not design drawings.



	Northern	Distrib	East
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SITES IN N	SITES IN NETWORK											
Site ID	CCG ID	Site Name										
1 01	NA	Farrell Road / Telopea Way										
101	NA	Telopea Way / Northern Distributor Road										

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Site: 101 [Farrell Road / Telopea Way (Site Folder: General)]

AM Peak - Strategic Model Volumes 2028 Site Category: (None) Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 50 seconds (Network Optimum Cycle Time -Minimum Delay)

Vehio	cle Mo	vement	Perfo	rmano	ce									
Mov ID	Turn	DEMA FLOV [Total veh/h	AND WS HV] %	ARRI FLO [Total veh/h	VAL WS HV] %	Deg. Satn v/c	Aver. Delay sec	Level of Service	AVERA OF C [Veh. veh	GE BACK QUEUE Dist] m	Prop. Que	EffectiveA Stop Rate	ver. No. Cycles	Aver. Speed km/h
South	: Telop	ea Way S	South											
2 3	T1 R2	146 174	3.0 3.0	146 174	3.0 3.0	0.120 * 0.398	3.6 14.7	LOS A LOS B	0.8 1.7	5.8 12.4	0.39 0.67	0.32 0.71	0.39 0.67	54.0 39.5
Appro	bach	320	3.0	320	3.0	0.398	9.6	LOS A	1.7	12.4	0.54	0.53	0.54	45.0
East:	Farrell	Road												
4	L2	161	3.0	161	3.0	0.261	14.2	LOS B	1.5	10.6	0.63	0.74	0.63	41.2
6	R2	21	3.0	21	3.0	0.096	28.0	LOS C	0.3	2.1	0.92	0.69	0.92	40.1
Appro	bach	182	3.0	182	3.0	0.261	15.8	LOS B	1.5	10.6	0.66	0.73	0.66	41.0
North	: Telop	ea Way N	lorth											
7	L2	31	3.0	31	3.0	* 0.140	28.3	LOS C	0.4	3.1	0.93	0.71	0.93	40.2
8	T1	143	3.0	143	3.0	*0.378	16.5	LOS B	1.8	13.1	0.85	0.69	0.85	39.1
Appro	bach	174	3.0	174	3.0	0.378	18.6	LOS B	1.8	13.1	0.86	0.69	0.86	39.4
All Ve	hicles	676	3.0	676	3.0	0.398	13.6	LOS B	1.8	13.1	0.66	0.63	0.66	42.2

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Geometric Delay is included).

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

* Critical Movement (Signal Timing)

Ped	Pedestrian Movement Performance														
Mov	Crossing	Dem.	Aver.	Level of	AVERAGE E	BACK OF	Prop. Ef	fective	Travel	Travel	Aver.				
ID	Crossing	Flow	Delay	Service	QUEUE [Ped Dist]		Que	Stop Rate	lime	Dist.	Speed				
		ped/h	sec		ped	m			sec	m	m/sec				
East	t: Farrell Road	ł													
P2	Full	53	19.4	LOS B	0.1	0.1	0.88	0.88	183.9	213.9	1.16				
Nort	h: Telopea Wa	ay North													
P3	Full	53	19.4	LOS B	0.1	0.1	0.88	0.88	183.9	213.9	1.16				
All Ped	estrians	105	19.4	LOS B	0.1	0.1	0.88	0.88	183.9	213.9	1.16				

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements. Organisation: AMBER ORGANISATION | Licence: PLUS / 1PC | Processed: Tuesday, 17 August 2021 10:31:28 AM Project: D:\OneDrive - Amber Organisation Pty Ltd\Amber\Jobs\213 - Leeds Parade, Clergate - Subdivision\SIDRA\Telopea - Farrell - NDR\213 - Telopea - Farrell - NDR 210817.sip9

Site: 101 [Farrell Road / Telopea Way (Site Folder: General)]

AM Peak - Future Volumes 2028 Site Category: (None) Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 50 seconds (Network Optimum Cycle Time -Minimum Delay)

Vehic	cle Mo	vement	Perfo	rmano	ce									
Mov ID	Turn	DEMA FLOV [Total veh/h	AND WS HV] %	ARRI FLO [Total veh/h	VAL WS HV] %	Deg. Satn v/c	Aver. Delay sec	Level of Service	AVERAO OF Q [Veh. veh	GE BACK UEUE Dist] m	Prop. Que	EffectiveA Stop Rate	ver. No. Cycles	Aver. Speed km/h
South	: Telop	ea Way S	South											
2 3	T1 R2	204 174	3.0 3.0	204 174	3.0 3.0	0.167 * 0.434	3.7 7.0	LOS A LOS A	1.2 0.7	8.4 5.4	0.40 0.29	0.34 0.60	0.40 0.29	53.8 47.0
Appro	bach	378	3.0	378	3.0	0.434	5.2	LOS A	1.2	8.4	0.35	0.46	0.35	50.5
East:	Farrell	Road												
4 6	L2 R2	192 78	3.0 3.0	192 78	3.0 3.0	0.365 * 0.357	15.4 29.2	LOS B	1.9 1 2	13.9 8.3	0.69 0.96	0.76 0.75	0.69 0.96	40.1 39.6
Appro	bach	269	3.0	269	3.0	0.365	19.4	LOS B	1.9	13.9	0.77	0.76	0.77	39.9
North	: Telop	ea Way N	lorth											
7	L2	45	3.0	45	3.0	0.207	28.6	LOS C	0.7	4.7	0.94	0.73	0.94	40.0
8	T1	158	3.0	158	3.0	*0.438	16.2	LOS B	2.0	14.5	0.85	0.70	0.85	39.3
Appro	bach	203	3.0	203	3.0	0.438	18.9	LOS B	2.0	14.5	0.87	0.71	0.87	39.6
All Ve	hicles	851	3.0	851	3.0	0.438	13.0	LOS B	2.0	14.5	0.61	0.61	0.61	43.4

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Geometric Delay is included).

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

* Critical Movement (Signal Timing)

Ped	Pedestrian Movement Performance														
Mov	Crossing	Dem.	Aver.	Level of	AVERAGE E	BACK OF	Prop. Ef	fective	Travel	Travel	Aver.				
ID	Crossing	Flow	Delay	Service	QUEUE [Ped Dist]		Que	Stop Rate	lime	Dist.	Speed				
		ped/h	sec		ped	m			sec	m	m/sec				
East	t: Farrell Road	ł													
P2	Full	53	19.4	LOS B	0.1	0.1	0.88	0.88	183.9	213.9	1.16				
Nort	h: Telopea Wa	ay North													
P3	Full	53	19.4	LOS B	0.1	0.1	0.88	0.88	183.9	213.9	1.16				
All Ped	estrians	105	19.4	LOS B	0.1	0.1	0.88	0.88	183.9	213.9	1.16				

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements. Organisation: AMBER ORGANISATION | Licence: PLUS / 1PC | Processed: Tuesday, 17 August 2021 10:31:38 AM Project: D:\OneDrive - Amber Organisation Pty Ltd\Amber\Jobs\213 - Leeds Parade, Clergate - Subdivision\SIDRA\Telopea - Farrell - NDR\213 - Telopea - Farrell - NDR 210817.sip9

Site: 101 [Farrell Road / Telopea Way (Site Folder: General)]

PM Peak - Strategic Model Volumes 2028 Site Category: (None) Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 40 seconds (Network Optimum Cycle Time -Minimum Delay)

Vehio	cle Mo	vement	Perfo	rmano	ce									
Mov ID	Turn	DEMA FLOV [Total veh/h	AND WS HV] %	ARRI FLO [Total veh/h	VAL WS HV] %	Deg. Satn v/c	Aver. Delay sec	Level of Service	AVERA OF C [Veh. veh	GE BACK QUEUE Dist] m	Prop. Que	EffectiveA Stop Rate	ver. No. Cycles	Aver. Speed km/h
South	: Telop	ea Way S	South											
2 3	T1 R2	332 123	3.0 3.0	332 123	3.0 3.0	0.315 * 0.452	4.3 20.9	LOS A LOS C	1.6 1.4	11.7 10.2	0.43 0.92	0.36 0.76	0.43 0.92	52.8 35.1
Appro	bach	455	3.0	455	3.0	0.452	8.8	LOS A	1.6	11.7	0.56	0.47	0.56	46.5
East:	Farrell	Road												
4 6	L2 R2	122 44	3.0 3.0	122 44	3.0 3.0	0.172 0.162	12.9 22.7	LOS B LOS C	0.9 0.5	6.4 3.6	0.63 0.90	0.72 0.72	0.63 0.90	42.4 42.6
Appro	bach	166	3.0	166	3.0	0.172	15.5	LOS B	0.9	6.4	0.70	0.72	0.70	42.5
North	: Telop	ea Way N	lorth											
7 8	L2 T1	47 245	3.0 3.0	47 245	3.0 3.0	*0.174 *0.592	22.8 15 5	LOS C	0.5 2.8	3.8 20.2	0.91 0.93	0.72 0.79	0.91 0.97	42.7 39 9
Appro	bach	293	3.0	293	3.0	0.592	16.7	LOS B	2.8	20.2	0.93	0.78	0.96	40.7
All Ve	hicles	914	3.0	914	3.0	0.592	12.6	LOS B	2.8	20.2	0.70	0.62	0.72	43.6

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Geometric Delay is included).

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

* Critical Movement (Signal Timing)

Pedestrian Mov	vement	Perform	nance							
Mov	Dem.	Aver.	Level of	AVERAGE E	BACK OF	Prop. Ef	fective	Travel	Travel	Aver.
ID Crossing	Flow	Delay	Service	QUEUE [Ped Dist]		Que	Stop Rate	Time	Dist.	Speed
	ped/h	sec		ped	m			sec	m	m/sec
East: Farrell Road	b									
P2 Full	53	14.5	LOS B	0.1	0.1	0.85	0.85	179.0	213.9	1.19
North: Telopea W	ay North									
P3 Full	53	14.5	LOS B	0.1	0.1	0.85	0.85	179.0	213.9	1.19
All Pedestrians	105	14.5	LOS B	0.1	0.1	0.85	0.85	179.0	213.9	1.19

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements. Organisation: AMBER ORGANISATION | Licence: PLUS / 1PC | Processed: Tuesday, 17 August 2021 10:31:46 AM Project: D:\OneDrive - Amber Organisation Pty Ltd\Amber\Jobs\213 - Leeds Parade, Clergate - Subdivision\SIDRA\Telopea - Farrell - NDR\213 - Telopea - Farrell - NDR 210817.sip9

Site: 101 [Farrell Road / Telopea Way (Site Folder: General)]

PM Peak - Future Volumes 2028 Site Category: (None) Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 50 seconds (Network Optimum Cycle Time -Minimum Delay)

Vehi	Vehicle Movement Performance														
Mov ID	Turn	DEMA FLO\ [Total veh/h	AND NS HV] %	ARRI FLO [Total veh/h	VAL WS HV] %	Deg. Satn v/c	Aver. Delay sec	Level of Service	AVERAC OF Q [Veh. veh	GE BACK UEUE Dist] m	Prop. Que	EffectiveA Stop Rate	ver. No. Cycles	Aver. Speed km/h	
South	n: Telop	ea Way S	South												
2	T1 R2	352 123	3.0 3.0	352 123	3.0 3.0	0.287	3.3 27 3	LOSA	1.6 1.9	11.6 13.6	0.33	0.28	0.33	54.4 31.4	
Appro	bach	475	3.0	475	3.0	0.564	9.5	LOS C	1.9	13.6	0.50	0.41	0.51	45.7	
East:	Farrell	Road													
4	L2	133	3.0	133	3.0	0.317	18.7	LOS B	1.5	10.8	0.76	0.76	0.76	37.4	
6	R2	64	3.0	64	3.0	0.294	28.9	LOS C	0.9	6.8	0.95	0.74	0.95	39.7	
Appro	bach	197	3.0	197	3.0	0.317	22.0	LOS C	1.5	10.8	0.82	0.76	0.82	38.5	
North	: Telope	ea Way N	lorth												
7	L2	93	3.0	93	3.0	*0.425	29.5	LOS C	1.4	10.0	0.97	0.76	0.97	39.6	
8	T1	292	3.0	292	3.0	*0.597	13.5	LOS B	3.6	26.1	0.84	0.72	0.85	41.7	
Appro	bach	384	3.0	384	3.0	0.597	17.3	LOS B	3.6	26.1	0.87	0.73	0.88	40.9	
All Ve	hicles	1056	3.0	1056	3.0	0.597	14.7	LOS B	3.6	26.1	0.69	0.59	0.70	42.1	

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Geometric Delay is included).

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

* Critical Movement (Signal Timing)

Pedestrian Movement Performance											
Mov	Crossing	Dem.	Aver.	Level of	of AVERAGE BACK OF		Prop. Effective		Travel	Travel	Aver.
ID	Crossing	Flow	Delay	Service	QUEU [Ped	JE Dist]	Que	Stop Rate	lime	Dist.	Speed
		ped/h	sec		ped	m			sec	m	m/sec
East: Farrell Road											
P2	Full	53	19.4	LOS B	0.1	0.1	0.88	0.88	183.9	213.9	1.16
North: Telopea Way North											
P3	Full	53	19.4	LOS B	0.1	0.1	0.88	0.88	183.9	213.9	1.16
All Pede	estrians	105	19.4	LOS B	0.1	0.1	0.88	0.88	183.9	213.9	1.16

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements. Organisation: AMBER ORGANISATION | Licence: PLUS / 1PC | Processed: Tuesday, 17 August 2021 10:31:56 AM Project: D:\OneDrive - Amber Organisation Pty Ltd\Amber\Jobs\213 - Leeds Parade, Clergate - Subdivision\SIDRA\Telopea - Farrell - NDR\213 - Telopea - Farrell - NDR 210817.sip9
Site: 101 [Telopea Way / Northern Distributor Road (Site Folder: General)]

Network: N101 [Telopea Way / Farrell Road / Northern Distributor Road (Network Folder: General)]

AM Peak - Strategic Model Volumes 2028 Site Category: (None) Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 50 seconds (Network Optimum Cycle Time -Minimum Delay)

Vehi	cle Mo	vement	Perfo	rmand	ce									
Mov ID	Turn	DEMA FLOV [Total veh/h	AND NS HV] %	ARRI FLO [Total veh/h	VAL WS HV] %	Deg. Satn v/c	Aver. Delay sec	Level of Service	AVERAC OF Q [Veh. veh	GE BACK UEUE Dist] m	Prop. Que	EffectiveA Stop Rate	ver. No. Cycles	Aver. Speed km/h
East:	Northe	rn Distrib	utor Ro	oad Ea	st									
5	T1	323	3.0	323	3.0	0.264	4.2	LOS A	2.1	15.1	0.46	0.39	0.46	56.1
6	R2	85	3.0	85	3.0	*0.406	29.4	LOS C	1.3	9.2	0.96	0.76	0.96	30.7
Appro	bach	408	3.0	408	3.0	0.406	9.5	LOS A	2.1	15.1	0.57	0.47	0.57	51.1
North	: Telope	ea Way												
7	L2	171	3.0	171	3.0	0.261	17.4	LOS B	2.2	16.0	0.90	0.79	0.90	37.5
9	R2	134	3.0	134	3.0	*0.613	27.6	LOS C	2.2	15.5	1.00	0.82	1.08	31.3
Appro	bach	304	3.0	304	3.0	0.613	21.9	LOS C	2.2	16.0	0.94	0.80	0.98	34.5
West	Northe	ern Distril	butor R	oad W	est									
10	L2	235	3.0	235	3.0	0.202	9.6	LOS A	1.5	10.5	0.44	0.69	0.44	45.8
11	T1	576	3.0	576	3.0	*0.753	16.3	LOS B	8.2	59.1	0.92	0.87	1.03	47.3
Appro	bach	811	3.0	811	3.0	0.753	14.4	LOS B	8.2	59.1	0.78	0.82	0.86	47.1
All Ve	hicles	1523	3.0	1523	3.0	0.753	14.6	LOS B	8.2	59.1	0.75	0.72	0.80	46.0

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Geometric Delay is included).

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

* Critical Movement (Signal Timing)

Pedestrian Mov	/ement	Perform	nance							
Mov ID Crossing	Dem. Flow	Aver. Delay	Level of Service	AVERAGE QUE	BACK OF	Prop. Ef Que	fective Stop	Travel Time	Travel Dist.	Aver. Speed
	ped/h	sec		l Ped ped	Dist j m		Rate	sec	m	m/sec
East: Northern Di	stributor	Road Ea	ast							
P2 Full	53	19.4	LOS B	0.1	0.1	0.88	0.88	183.9	213.9	1.16
North: Telopea W	ay									
P3 Full	53	19.4	LOS B	0.1	0.1	0.88	0.88	186.5	217.2	1.16
West: Northern D	istributor	^r Road V	Vest							
P4 Full	53	19.4	LOS B	0.1	0.1	0.88	0.88	183.9	213.9	1.16
All Pedestrians	158	19.4	LOS B	0.1	0.1	0.88	0.88	184.8	215.0	1.16

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Site: 101 [Telopea Way / Northern Distributor Road (Site Folder: General)]

AM Peak - Future Volumes 2028 Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 50 seconds (Network Optimum Cycle Time - Minimum Delay)

Vehi	cle Mo	vement	Perfo	rmano	ce									
Mov ID	Turn	DEMA FLOV [Total veh/h	AND WS HV] %	ARRI FLO [Total veh/h	VAL WS HV] %	Deg. Satn v/c	Aver. Delay sec	Level of Service	AVERAO OF Q [Veh. veh	GE BACK UEUE Dist] m	Prop. Que	EffectiveA Stop Rate	ver. No. Cycles	Aver. Speed km/h
East:	Northe	rn Distrib	outor Re	oad Ea	st									
5	T1	376	3.0	376	3.0	0.307	4.4	LOS A	2.5	18.2	0.48	0.41	0.48	56.0
6	R2	143	3.0	143	3.0	*0.656	31.0	LOS C	2.3	16.5	1.00	0.85	1.16	29.9
Appro	bach	519	3.0	519	3.0	0.656	11.7	LOS B	2.5	18.2	0.62	0.53	0.67	49.1
North	: Telop	ea Way												
7	L2	184	3.0	184	3.0	0.281	17.6	LOS B	2.4	17.4	0.91	0.79	0.91	37.4
9	R2	164	3.0	164	3.0	*0.753	30.6	LOS C	2.7	19.6	1.00	0.86	1.16	29.8
Appro	bach	348	3.0	348	3.0	0.753	23.7	LOS C	2.7	19.6	0.95	0.82	1.02	33.4
West	Northe	ern Distri	butor R	oad W	est									
10	L2	235	3.0	235	3.0	0.202	9.6	LOS A	1.5	10.5	0.44	0.69	0.44	45.8
11	T1	597	3.0	597	3.0	*0.780	17.5	LOS B	8.9	63.9	0.93	0.91	1.08	46.6
Appro	bach	832	3.0	832	3.0	0.780	15.2	LOS B	8.9	63.9	0.79	0.85	0.90	46.5
All Ve	hicles	1699	3.0	1699	3.0	0.780	15.9	LOS B	8.9	63.9	0.77	0.75	0.85	44.9

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Geometric Delay is included).

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

* Critical Movement (Signal Timing)

Pedestrian Mov	/ement	Perform	nance							
Mov ID Crossing	Dem. Flow	Aver. Delay	Level of Service	AVERAGE QUE	BACK OF	Prop. Ef Que	fective Stop	Travel Time	Travel Dist.	Aver. Speed
	ped/h	sec		l Ped ped	Dist j m		Rate	sec	m	m/sec
East: Northern Di	stributor	Road Ea	ast							
P2 Full	53	19.4	LOS B	0.1	0.1	0.88	0.88	183.9	213.9	1.16
North: Telopea W	ay									
P3 Full	53	19.4	LOS B	0.1	0.1	0.88	0.88	186.5	217.2	1.16
West: Northern D	istributor	^r Road V	Vest							
P4 Full	53	19.4	LOS B	0.1	0.1	0.88	0.88	183.9	213.9	1.16
All Pedestrians	158	19.4	LOS B	0.1	0.1	0.88	0.88	184.8	215.0	1.16

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Site: 101 [Telopea Way / Northern Distributor Road (Site Folder: General)]

PM Peak - Strategic Model Volumes 2028 Site Category: (None) Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 40 seconds (Network Optimum Cycle Time -Minimum Delay)

Vehi	cle Mo	vement	Perfo	rmano	ce									
Mov ID	Turn	DEMA FLOV [Total veh/h	AND WS HV] %	ARRI FLO [Total veh/h	VAL WS HV] %	Deg. Satn v/c	Aver. Delay sec	Level of Service	AVERAO OF C [Veh. veh	GE BACK QUEUE Dist] m	Prop. Que	EffectiveA Stop Rate	ver. No. Cycles	Aver. Speed km/h
East:	Northe	rn Distrib	outor Re	oad Ea	st									
5 6	T1 R2	624 208	3.0 3.0	624 208	3.0 3.0	0.593 * 0.764	6.6 27.0	LOS A LOS C	5.1 2.8	36.7 20.3	0.72 1.00	0.63 0.94	0.72 1.35	54.1 32.0
Appro	bach	833	3.0	833	3.0	0.764	11.7	LOS B	5.1	36.7	0.79	0.71	0.87	49.2
North	: Telope	ea Way												
7 9	L2 R2	205 163	3.0 3.0	205 163	3.0 3.0	0.251 * 0.598	11.4 19.8	LOS B LOS B	1.9 1.9	13.4 13.8	0.78 0.93	0.76 0.80	0.78 0.98	42.5 35.9
Appro	bach	368	3.0	368	3.0	0.598	15.1	LOS B	1.9	13.8	0.85	0.77	0.87	39.3
West	: Northe	ern Distri	butor R	oad W	est									
10 11	L2 T1	247 342	3.0 3.0	247 342	3.0 3.0	0.250 * 0.715	10.7 17.1	LOS B LOS B	1.6 4.2	11.3 30.3	0.55 0.97	0.72 0.89	0.55 1.13	44.6 46.8
Appro	bach	589	3.0	589	3.0	0.715	14.4	LOS B	4.2	30.3	0.79	0.82	0.89	46.2
All Ve	hicles	1791	3.0	1791	3.0	0.764	13.3	LOS B	5.1	36.7	0.80	0.76	0.88	46.5

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Geometric Delay is included).

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

* Critical Movement (Signal Timing)

Pedestrian Mo	vement	Perform	nance							
Mov ID Crossing	Dem. Flow	Aver. Delay	Level of Service	AVERAGE QUE	BACK OF	Prop. El Que	fective Stop	Travel Time	Travel Dist.	Aver. Speed
	ped/h	sec		[Ped ped	Dist] m		Rate	sec	m	m/sec
East: Northern D	istributor	Road Ea	ast							
P2 Full	53	14.5	LOS B	0.1	0.1	0.85	0.85	179.0	213.9	1.19
North: Telopea W	/ay									
P3 Full	53	14.5	LOS B	0.1	0.1	0.85	0.85	181.6	217.2	1.20
West: Northern D	Distributo	r Road V	Vest							
P4 Full	53	14.5	LOS B	0.1	0.1	0.85	0.85	179.0	213.9	1.19
All Pedestrians	158	14.5	LOS B	0.1	0.1	0.85	0.85	179.9	215.0	1.20

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Site: 101 [Telopea Way / Northern Distributor Road (Site Folder: General)]

■ Network: N101 [Telopea Way / Farrell Road / Northern Distributor Road (Network Folder: General)]

PM Peak - Future Volumes 2028 Site Category: (None) Signals - EQUISAT (Fixed-Time/SCATS) Co

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 50 seconds (Network Optimum Cycle Time - Minimum Delay)

Vehio	cle Mo	vement	Perfo	rmand	ce									
Mov ID	Turn	DEMA FLO\ [Total veh/h	AND NS HV] %	ARRI FLO [Total veh/h	VAL WS HV] %	Deg. Satn v/c	Aver. Delay sec	Level of Service	AVERAO OF C [Veh. veh	GE BACK UEUE Dist] m	Prop. Que	EffectiveA Stop Rate	ver. No. Cycles	Aver. Speed km/h
East:	Northe	rn Distrib	utor Ro	oad Ea	st									
5 6	T1 R2	642 228	3.0 3.0	642 228	3.0 3.0	0.541 * 0.736	5.9 29.4	LOS A LOS C	5.6 3.7	39.9 26.4	0.61 0.99	0.55 0.91	0.61 1.22	54.7 30.7
Appro	bach	871	3.0	871	3.0	0.736	12.1	LOS B	5.6	39.9	0.71	0.64	0.77	48.9
North	: Telope	ea Way												
7 9	L2 R2	252 174	3.0 3.0	252 174	3.0 3.0	0.301 * 0.682	12.0 20.0	LOS B	2.4 2.4	17.2 17.0	0.66 0.88	0.73 0.78	0.66 0.92	42.0 35.7
Appro	bach	425	3.0	425	3.0	0.682	15.3	LOS B	2.4	17.2	0.75	0.75	0.77	39.1
West:	Northe	ern Distril	butor R	oad W	est									
10 11	L2 T1	247 408	3.0 3.0	247 408	3.0 3.0	0.243 * 0.712	11.7 18.7	LOS B LOS B	1.9 5.9	13.7 42.6	0.54 0.95	0.72 0.86	0.54 1.04	43.6 45.9
Appro	bach	656	3.0	656	3.0	0.712	16.1	LOS B	5.9	42.6	0.79	0.81	0.85	45.3
All Ve	hicles	1952	3.0	1952	3.0	0.736	14.1	LOS B	5.9	42.6	0.75	0.72	0.80	45.9

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Geometric Delay is included).

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

* Critical Movement (Signal Timing)

Pedestrian Mov	/ement	Perform	nance							
Mov ID Crossing	Dem. Flow	Aver. Delay	Level of Service	AVERAGE QUE	BACK OF	Prop. Ef Que	fective Stop	Travel Time	Travel Dist.	Aver. Speed
	ped/h	sec		l Ped ped	Dist j m		Rate	sec	m	m/sec
East: Northern Di	stributor	Road Ea	ast							
P2 Full	53	19.4	LOS B	0.1	0.1	0.88	0.88	183.9	213.9	1.16
North: Telopea W	ay									
P3 Full	53	19.4	LOS B	0.1	0.1	0.88	0.88	186.5	217.2	1.16
West: Northern D	istributor	^r Road V	Vest							
P4 Full	53	19.4	LOS B	0.1	0.1	0.88	0.88	183.9	213.9	1.16
All Pedestrians	158	19.4	LOS B	0.1	0.1	0.88	0.88	184.8	215.0	1.16

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SITE LAYOUT V Site: 101 [Clergate Road / Site Access (Site Folder: General)]

PM Peak - 2028 Future Traffic Volumes Site Category: (None) Give-Way (Two-Way)

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V Site: 101 [Clergate Road / Site Access (Site Folder: General)]

AM Peak - 2028 Future Traffic Volumes Site Category: (None) Give-Way (Two-Way)

Vehi	cle M	ovemen	t Perfor	mance										
Mov ID	Turn	INF VOLL		DEM/ FLO	AND WS	Deg. Satn	Aver. Delay	Level of Service	95% BA QUE	ACK OF EUE Dist 1	Prop. Que	Effective Stop	Aver. No.	Aver. Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m		Trate	Cycles	km/h
South	n: Cler	gate Roa	d South											
2	T1	8	3.0	8	3.0	0.022	0.1	LOS A	0.1	0.7	0.12	0.44	0.12	55.7
3	R2	28	3.0	29	3.0	0.022	5.6	LOS A	0.1	0.7	0.12	0.44	0.12	53.6
Appro	oach	36	3.0	38	3.0	0.022	4.4	NA	0.1	0.7	0.12	0.44	0.12	54.1
East:	Site A	ccess												
4	L2	111	3.0	117	3.0	0.092	5.7	LOS A	0.4	2.7	0.11	0.55	0.11	53.2
6	R2	20	3.0	21	3.0	0.092	5.8	LOS A	0.4	2.7	0.11	0.55	0.11	52.6
Appro	bach	131	3.0	138	3.0	0.092	5.7	LOS A	0.4	2.7	0.11	0.55	0.11	53.1
North	: Clerç	gate Roa	d North											
7	L2	5	3.0	5	3.0	0.021	5.6	LOS A	0.0	0.0	0.00	0.08	0.00	57.5
8	T1	33	3.0	35	3.0	0.021	0.0	LOS A	0.0	0.0	0.00	0.08	0.00	59.3
Appro	bach	38	3.0	40	3.0	0.021	0.7	NA	0.0	0.0	0.00	0.08	0.00	59.0
All Vehic	les	205	3.0	216	3.0	0.092	4.5	NA	0.4	2.7	0.09	0.44	0.09	54.3

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

Delay Model: SIDRA Standard (Geometric Delay is included).

Queue Model: SIDRA Standard.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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V Site: 101 [Clergate Road / Site Access (Site Folder: General)]

PM Peak - 2028 Future Traffic Volumes Site Category: (None) Give-Way (Two-Way)

Vehi	cle M	ovemen	t Perfo	mance										
Mov	Turn	INP	TU	DEM/	AND	Deg.	Aver.	Level of	95% BA	ACK OF	Prop.	Effective	Aver.	Aver.
ID		VOLL	IMES	FLO'	WS	Satn	Delay	Service	QUE	EUE	Que	Stop	No.	Speed
		[Total	HV]	[Total	HV]				[Veh.	Dist]		Rate	Cycles	
		veh/h	%	veh/h	%	V/C	sec		veh	m				km/h
Sout	h: Cler	gate Roa	d South											
2	T1	27	3.0	28	3.0	0.070	0.1	LOS A	0.3	2.4	0.10	0.44	0.10	55.8
3	R2	89	3.0	94	3.0	0.070	5.6	LOS A	0.3	2.4	0.10	0.44	0.10	53.7
Appr	oach	116	3.0	122	3.0	0.070	4.3	NA	0.3	2.4	0.10	0.44	0.10	54.2
East:	Site A	ccess												
4	L2	38	3.0	40	3.0	0.032	5.6	LOS A	0.1	0.9	0.04	0.57	0.04	53.4
6	R2	7	3.0	7	3.0	0.032	6.0	LOS A	0.1	0.9	0.04	0.57	0.04	52.9
Appr	oach	45	3.0	47	3.0	0.032	5.7	LOS A	0.1	0.9	0.04	0.57	0.04	53.3
North	n: Clerg	gate Roa	d North											
7	L2	16	3.0	17	3.0	0.015	5.6	LOS A	0.0	0.0	0.00	0.36	0.00	55.2
8	T1	10	3.0	11	3.0	0.015	0.0	LOS A	0.0	0.0	0.00	0.36	0.00	56.8
Appr	oach	26	3.0	27	3.0	0.015	3.4	NA	0.0	0.0	0.00	0.36	0.00	55.8
All Vehic	cles	187	3.0	197	3.0	0.070	4.5	NA	0.3	2.4	0.07	0.46	0.07	54.2

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

Delay Model: SIDRA Standard (Geometric Delay is included).

Queue Model: SIDRA Standard.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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SITE LAYOUT V Site: 101 [Clergate Road / Pearce Lane / Site Access (Site Folder: General)]

AM Peak - 2028 Orange Traffic Model Volumes Site Category: Future Conditions 1 Give-Way (Two-Way)

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V Site: 101 [Clergate Road / Pearce Lane / Site Access (Site Folder: General)]

AM Peak - 2028 Orange Traffic Model Volumes Site Category: Future Conditions 1 Give-Way (Two-Way)

Vehi	cle M	ovemen	t Perfo	rmance										
Mov	Turn	INP	UT	DEM/	AND	Deg.	Aver.	Level of	95% BA	ACK OF	Prop.	Effective	Aver.	Aver.
				FLO'	WS	Satn	Delay	Service		EUE	Que	Stop	No.	Speed
		veh/h	HV J %	veh/h	нvј %	v/c	sec		ven. veh	m Dist		Rale	Cycles	km/h
South	n: Site	Access												
1	L2	1	5.0	1	5.0	0.002	5.6	LOS A	0.0	0.1	0.08	0.54	0.08	53.6
2	T1	1	5.0	1	5.0	0.002	4.3	LOS A	0.0	0.1	0.08	0.54	0.08	53.8
3	R2	1	5.0	1	5.0	0.002	5.6	LOS A	0.0	0.1	0.08	0.54	0.08	53.0
Appro	oach	3	5.0	3	5.0	0.002	5.2	LOS A	0.0	0.1	0.08	0.54	0.08	53.4
East:	Pearo	e Lane E	ast											
4	L2	1	5.0	1	5.0	0.014	5.6	LOS A	0.0	0.4	0.03	0.53	0.03	54.3
5	T1	15	5.0	16	5.0	0.014	4.2	LOS A	0.0	0.4	0.03	0.53	0.03	54.5
6	R2	1	5.0	1	5.0	0.014	5.6	LOS A	0.0	0.4	0.03	0.53	0.03	53.7
Appro	oach	17	5.0	18	5.0	0.014	4.4	LOS A	0.0	0.4	0.03	0.53	0.03	54.4
North	: Cler	gate Roa	d											
7	L2	1	5.0	1	5.0	0.002	5.6	LOS A	0.0	0.0	0.00	0.40	0.00	54.9
8	T1	1	5.0	1	5.0	0.002	0.0	LOS A	0.0	0.0	0.00	0.40	0.00	56.6
9	R2	1	5.0	1	5.0	0.002	5.5	LOS A	0.0	0.0	0.00	0.40	0.00	54.3
Appro	oach	3	5.0	3	5.0	0.002	3.7	NA	0.0	0.0	0.00	0.40	0.00	55.3
West	: Pear	ce Lane												
10	L2	2	5.0	2	5.0	0.007	5.6	LOS A	0.0	0.3	0.03	0.20	0.03	56.3
11	T1	6	5.0	6	5.0	0.007	0.0	LOS A	0.0	0.3	0.03	0.20	0.03	58.1
12	R2	1	5.0	1	5.0	0.007	5.5	LOS A	0.0	0.3	0.03	0.20	0.03	55.7
Appro	oach	9	5.0	9	5.0	0.007	1.9	NA	0.0	0.3	0.03	0.20	0.03	57.4
All Vehic	les	32	5.0	34	5.0	0.014	3.7	NA	0.0	0.4	0.03	0.42	0.03	55.2

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

Delay Model: SIDRA Standard (Geometric Delay is included).

Queue Model: SIDRA Standard.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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V Site: 101 [Clergate Road / Pearce Lane / Site Access (Site Folder: General)]

AM Peak - 2028 Total Volumes Site Category: Future Conditions 1 Give-Way (Two-Way)

Vehi	cle M	ovemen	t Perfo	rmance										
Mov	Turn	INP	UT	DEMA		Deg.	Aver.	Level of	95% BA	CK OF	Prop. E	ffective	Aver.	Aver.
ID		VOLU		FLO\ []	WS	Satn	Delay	Service	QUE [\/ob	UE Dict 1	Que	Stop	No.	Speed
		veh/h	пvј %	veh/h	пvј %	v/c	sec		veh	m m		Nale	Cycles	km/h
Sout	h: Site	Access												
1	L2	22	5.0	23	5.0	0.034	5.7	LOS A	0.1	0.9	0.08	0.53	0.08	53.7
2	T1	22	5.0	23	5.0	0.034	4.4	LOS A	0.1	0.9	0.08	0.53	0.08	53.9
3	R2	1	5.0	1	5.0	0.034	5.7	LOS A	0.1	0.9	0.08	0.53	0.08	53.1
Appr	oach	45	5.0	47	5.0	0.034	5.0	LOS A	0.1	0.9	0.08	0.53	0.08	53.8
East:	Pearc	e Lane E	ast											
4	L2	1	5.0	1	5.0	0.014	5.6	LOS A	0.0	0.4	0.06	0.52	0.06	54.2
5	T1	15	5.0	16	5.0	0.014	4.2	LOS A	0.0	0.4	0.06	0.52	0.06	54.4
6	R2	1	5.0	1	5.0	0.014	5.8	LOS A	0.0	0.4	0.06	0.52	0.06	53.6
Appr	oach	17	5.0	18	5.0	0.014	4.4	LOS A	0.0	0.4	0.06	0.52	0.06	54.3
North	n: Clerg	gate Road	b											
7	L2	1	5.0	1	5.0	0.006	5.6	LOS A	0.0	0.0	0.00	0.30	0.00	55.7
8	T1	5	5.0	5	5.0	0.006	0.0	LOS A	0.0	0.0	0.00	0.30	0.00	57.4
9	R2	4	5.0	4	5.0	0.006	5.5	LOS A	0.0	0.0	0.00	0.30	0.00	55.1
Appr	oach	10	5.0	11	5.0	0.006	2.8	NA	0.0	0.0	0.00	0.30	0.00	56.3
West	: Pear	ce Lane												
10	L2	16	5.0	17	5.0	0.018	5.6	LOS A	0.1	0.5	0.05	0.44	0.05	54.2
11	T1	6	5.0	6	5.0	0.018	0.1	LOS A	0.1	0.5	0.05	0.44	0.05	55.9
12	R2	5	5.0	5	5.0	0.018	5.5	LOS A	0.1	0.5	0.05	0.44	0.05	53.7
Appr	oach	27	5.0	28	5.0	0.018	4.4	NA	0.1	0.5	0.05	0.44	0.05	54.5
All Vehic	cles	99	5.0	104	5.0	0.034	4.5	NA	0.1	0.9	0.06	0.48	0.06	54.3

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

Delay Model: SIDRA Standard (Geometric Delay is included).

Queue Model: SIDRA Standard.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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V Site: 101 [Clergate Road / Pearce Lane / Site Access (Site Folder: General)]

PM Peak - 2028 Orange Traffic Model Volumes Site Category: Future Conditions 1 Give-Way (Two-Way)

Vehicle Movement Performance														
Mov Turn		INPUT		DEMAND		Deg.	Aver. Level of		95% BACK OF		Prop. Effective		Aver.	Aver.
		VOLUMES		FLOWS		Satn	Delay Servic		QUEUE		Que Stop		No.	Speed
		l Iotai veh/h	нvј %	l Iotai veh/h	HVJ %	v/c	sec		ر ven. veh	Dist j m		Rate	Cycles	km/h
South	n: Site	Access												
1	L2	1	5.0	1	5.0	0.002	5.6	LOS A	0.0	0.1	0.04	0.55	0.04	53.7
2	T1	1	5.0	1	5.0	0.002	4.3	LOS A	0.0	0.1	0.04	0.55	0.04	53.9
3	R2	1	5.0	1	5.0	0.002	5.6	LOS A	0.0	0.1	0.04	0.55	0.04	53.1
Appro	oach	3	5.0	3	5.0	0.002	5.2	LOS A	0.0	0.1	0.04	0.55	0.04	53.5
East: Pearce Lane East														
4	L2	1	5.0	1	5.0	0.005	5.6	LOS A	0.0	0.1	0.02	0.54	0.02	54.1
5	T1	4	5.0	4	5.0	0.005	4.2	LOS A	0.0	0.1	0.02	0.54	0.02	54.3
6	R2	1	5.0	1	5.0	0.005	5.6	LOS A	0.0	0.1	0.02	0.54	0.02	53.5
Appro	oach	6	5.0	6	5.0	0.005	4.7	LOS A	0.0	0.1	0.02	0.54	0.02	54.1
North: Clergate Road														
7	L2	1	5.0	1	5.0	0.002	5.6	LOS A	0.0	0.0	0.00	0.40	0.00	54.9
8	T1	1	5.0	1	5.0	0.002	0.0	LOS A	0.0	0.0	0.00	0.40	0.00	56.6
9	R2	1	5.0	1	5.0	0.002	5.5	LOS A	0.0	0.0	0.00	0.40	0.00	54.3
Appro	oach	3	5.0	3	5.0	0.002	3.7	NA	0.0	0.0	0.00	0.40	0.00	55.3
West: Pearce Lane														
10	L2	1	5.0	1	5.0	0.013	5.6	LOS A	0.1	0.4	0.03	0.08	0.03	57.3
11	T1	13	5.0	14	5.0	0.013	0.0	LOS A	0.1	0.4	0.03	0.08	0.03	59.1
12	R2	1	5.0	1	5.0	0.013	5.5	LOS A	0.1	0.4	0.03	0.08	0.03	56.7
Appro	oach	15	5.0	16	5.0	0.013	0.8	NA	0.1	0.4	0.03	0.08	0.03	58.8
All Vehic	les	27	5.0	28	5.0	0.013	2.4	NA	0.1	0.4	0.03	0.27	0.03	56.7

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

Delay Model: SIDRA Standard (Geometric Delay is included).

Queue Model: SIDRA Standard.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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V Site: 101 [Clergate Road / Pearce Lane / Site Access - (Site Folder: General)]

PM Peak - 2028 Total Volumes Site Category: Future Conditions 1 Give-Way (Two-Way)

Vehicle Movement Performance														
Mov Turn		INPUT		DEMAND		Deg.	Aver. Level of		95% BACK OF		Prop. Effective		Aver.	Aver.
ID		VOLU	MES	FLO\	WS	Satn	Delay	Service	QUE		Que	Stop	No.	Speed
		veh/h	пvј %	veh/h	⊓vj %	v/c	sec		ven. veh	m Dist		Rale	Cycles	km/h
South: Site Access														
1	L2	7	5.0	7	5.0	0.012	5.6	LOS A	0.0	0.3	0.08	0.53	0.08	53.7
2	T1	7	5.0	7	5.0	0.012	4.4	LOS A	0.0	0.3	0.08	0.53	0.08	53.9
3	R2	1	5.0	1	5.0	0.012	5.8	LOS A	0.0	0.3	0.08	0.53	0.08	53.1
Appr	oach	15	5.0	16	5.0	0.012	5.1	LOS A	0.0	0.3	0.08	0.53	0.08	53.7
East: Pearce Lane East														
4	L2	1	5.0	1	5.0	0.005	5.7	LOS A	0.0	0.1	0.12	0.52	0.12	53.8
5	T1	4	5.0	4	5.0	0.005	4.4	LOS A	0.0	0.1	0.12	0.52	0.12	54.0
6	R2	1	5.0	1	5.0	0.005	5.8	LOS A	0.0	0.1	0.12	0.52	0.12	53.2
Appr	oach	6	5.0	6	5.0	0.005	4.8	LOS A	0.0	0.1	0.12	0.52	0.12	53.8
North: Clergate Road														
7	L2	1	5.0	1	5.0	0.017	5.6	LOS A	0.0	0.0	0.00	0.26	0.00	56.0
8	T1	17	5.0	18	5.0	0.017	0.0	LOS A	0.0	0.0	0.00	0.26	0.00	57.7
9	R2	12	5.0	13	5.0	0.017	5.5	LOS A	0.0	0.0	0.00	0.26	0.00	55.4
Appr	oach	30	5.0	32	5.0	0.017	2.4	NA	0.0	0.0	0.00	0.26	0.00	56.7
West: Pearce Lane														
10	L2	6	5.0	6	5.0	0.026	5.7	LOS A	0.1	0.9	0.12	0.36	0.12	54.7
11	T1	13	5.0	14	5.0	0.026	0.2	LOS A	0.1	0.9	0.12	0.36	0.12	56.3
12	R2	17	5.0	18	5.0	0.026	5.6	LOS A	0.1	0.9	0.12	0.36	0.12	54.1
Appr	oach	36	5.0	38	5.0	0.026	3.7	NA	0.1	0.9	0.12	0.36	0.12	55.0
All Vehic	cles	87	5.0	92	5.0	0.026	3.6	NA	0.1	0.9	0.07	0.37	0.07	55.3

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

Delay Model: SIDRA Standard (Geometric Delay is included).

Queue Model: SIDRA Standard.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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